

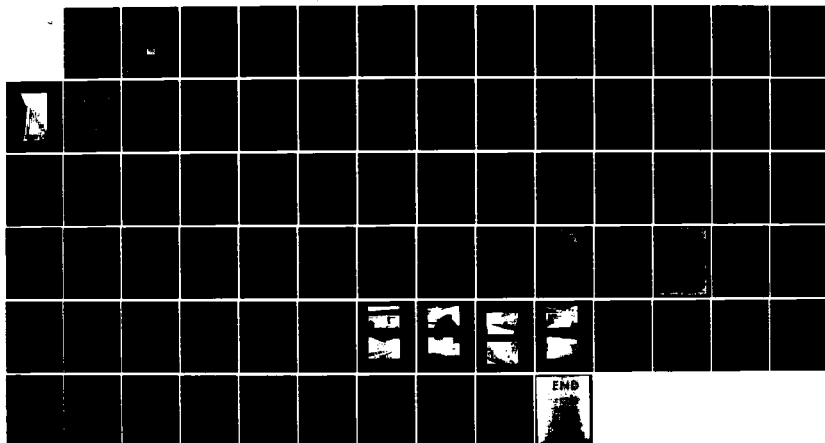
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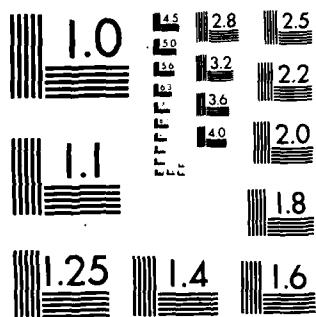
NATIONAL PROGRAM FOR INSPECTION OF NON-FEDERAL DAMS
FACTORY POND DAM (CT.) (U) CORPS OF ENGINEERS WALTHAM MA
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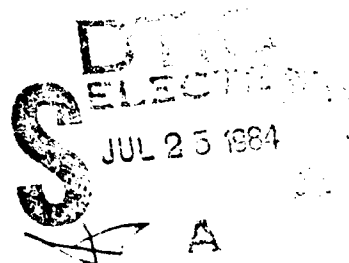
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NORWALK RIVER BASIN
GEORGETOWN CONNECTICUT

FACTORY POND DAM CT 00217

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASS. 02154

JULY 1980

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| 19. KEY WORDS (Continue on reverse side if necessary and identify by block number) DAMS, INSPECTION, DAM SAFETY, Norwalk River Basin Georgetown Conn. Factory Pond Dam | | |
| 20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Factory Pond Dam is a combination of sheet piling/concrete fill and masonry that is approx. 175 ft. long and 18.75 ft. high. The sheet piling portion of the dam consists of two rows of piling, 5 ft. apart, filled with concrete. The assess- ment of the dam is based on the visual inspection, past operational performance and hydraulic/hydrologic computations. The dam is judged to be in fair condition with several areas that require attention. The dam is classified as small and has a high hazard potential in accordance with guidelines established by the Corps of Engineers. | | |



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
424 TRAPELO ROAD
WALTHAM, MASSACHUSETTS 02254

REPLY TO
ATTENTION OF:

NEDED-E

NOV 14 1980

Honorable Ella T. Grasso
Governor of the State of Connecticut
State Capitol
Hartford, Connecticut 06115

Dear Governor Grasso:

Inclosed is a copy of the Factory Pond Dam (CT-00217) Phase I Inspection Report, which was prepared under the National Program for Inspection of Non-Federal Dams. The report is based upon a visual inspection, a review of past performance, and a preliminary hydrological analysis. A brief assessment is included at the beginning of the report.

The preliminary hydrologic analysis has indicated that the spillway capacity for the Factory Pond Dam would likely be exceeded by floods greater than 15 percent of the Probable Maximum Flood (PMF), the test flood for spillway adequacy. Our screening criteria specifies that a dam of this class which does not have sufficient spillway capacity to discharge fifty percent of the PMF, should be adjudged as having a seriously inadequate spillway and the dam assessed as unsafe, non-emergency, until more detailed studies prove otherwise or corrective measures are completed.

The term "unsafe" applied to a dam because of an inadequate spillway does not indicate the same degree of emergency as that term would if applied because of structural deficiency. It does indicate, however, that a severe storm may cause overtopping and possible failure of the dam, with significant damage and potential loss of life downstream.

It is recommended that within twelve months from the date of this report the owner of the dam engage the services of a professional or consulting engineer to determine by more sophisticated methods and procedures the magnitude of the spillway deficiency. Based on this determination, appropriate remedial mitigating measures should be designed and completed within 24 months of this date of notification. In the interim a detailed emergency operation plan and warning system should be promptly developed. During periods of unusually heavy precipitation, round-the-clock surveillance should be provided.

NOV 14 1980

NEDED-E

Honorable Ella T. Grasso

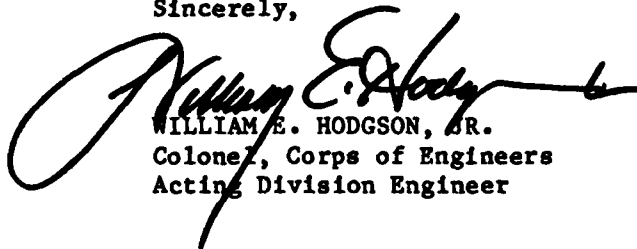
I have approved the report and support the findings and recommendations described in Section 7, with qualifications as noted above. I request that you keep me informed of the actions taken to implement these recommendations since this follow-up is an important part of the non-Federal Dam Inspection Program.

A copy of this report has been forwarded to the Department of Environmental Protection, the cooperating agency for the State of Connecticut. This report has also been furnished to the owner of the project, Gilbert & Bennett, Georgetown, Conn.

Copies of this report will be made available to the public, upon request to this office, under the Freedom of Information Act, thirty days from the date of this letter.

I wish to take this opportunity to thank you and the Department of Environmental Protection for the cooperation extended in carrying out this program.

Sincerely,


WILLIAM E. HODGSON, JR.
Colonel, Corps of Engineers
Acting Division Engineer



FACTORY POND DAM

CT 00217

NORWALK RIVER BASIN
GEORGETOWN, CONNECTICUT

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM

NATIONAL DAM INSPECTION PROGRAM

PHASE I INSPECTION REPORT

| | |
|------------------------|-------------------------------|
| Identification Number: | CT 00217 |
| Name: | Factory Pond Dam |
| Town: | Redding |
| County and State: | Fairfield County, Connecticut |
| Stream: | Norwalk River |
| Date of Inspection: | April 23, 1980 |

BRIEF ASSESSMENT

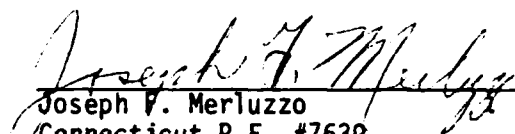
Factory Pond Dam is a combination of sheet piling/concrete fill and masonry that is approximately 175 feet long and 18.75 feet high. The sheet piling portion of the dam consists of two rows of piling, 5 feet apart, filled with concrete. The spillway is located on the southern portion of the dam and consists of a 75-foot long masonry weir. There is a 5-foot diameter discharge pipe that passes through the dam and an adjacent factory. Inside this pipe is a turbine that was once used for water power, but is now only used as a valve. The drainage area is 12.2 square miles and the reservoir has 192 acre-feet of available storage.


The assessment of the dam is based on the visual inspection, past operational performance and hydraulic/hydrologic computations. The dam is judged to be in fair condition with several areas that require attention. These areas include seepage in the vicinity of the west abutment of the spillway, concrete that needs repairing and masonry that needs repointing.

The dam is classified as small and has a high hazard potential in accordance with guidelines established by the Corps of Engineers. The test flood for this dam is 1/2 the Probable Maximum Flood (PMF). The test flood inflow is 9,640 cfs and the routed test flood outflow is 8,250 cfs. The test flood will overtop the dam by 4.25 feet.

It is recommended that the owner engage the services of a qualified registered engineer experienced in the design of dams to investigate the seepage through the dam and prepare a detailed hydraulic/hydrologic study to determine the spillway's adequacy. It is also recommended that the owner repair all cracks and mortared joints in the masonry portion of the dam; remove vegetation from the toe of the dam; establish a formal warning system; and initiate an annual technical inspection.

The owner should implement the recommendations and remedial measures described above and in greater detail in Section 7 within one year after receipt of this Phase I Inspection Report.


Joseph F. Merluzzo
Connecticut P.E. #7639
Project Manager


Gary J. Givoux
Connecticut P.E. #11477
Project Engineer

This Phase I Inspection Report on Factory Pond Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and is hereby submitted for approval.

Aramast Mahtesian

ARAMAST MAHTESIAN, MEMBER
Geotechnical Engineering Branch
Engineering Division

Carney M. Terzian

CARNEY M. TERZIAN, MEMBER
Design Branch
Engineering Division

Richard J. DiBuono

RICHARD DIBUONO, CHAIRMAN
Water Control Branch
Engineering Division

APPROVAL RECOMMENDED:

Joe B. Fryar

JOE B. FRYAR
Chief, Engineering Division

PREFACE

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Inspections. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Inspection is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigations and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I Inspection; however, the investigation is intended to identify any need for such studies.

In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I Inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established guidelines, the Spillway Test Flood is based on the estimated Probable Maximum Flood for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and variety of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aide in determining the need for more detailed hydrologic and hydraulic studies considering the size of the dam, its general condition and the downstream damage potential.

The Phase I Inspection does not include an assessment of the need for fences, gates, "no trespassing" signs, repairs to existing fences and railings and other items which may be needed to minimize trespass and provide greater security for the facility and safety to the public. An evaluation of the project for compliance with Occupational Safety and Hazard Administration's (OSHA) rules and regulations is also excluded.

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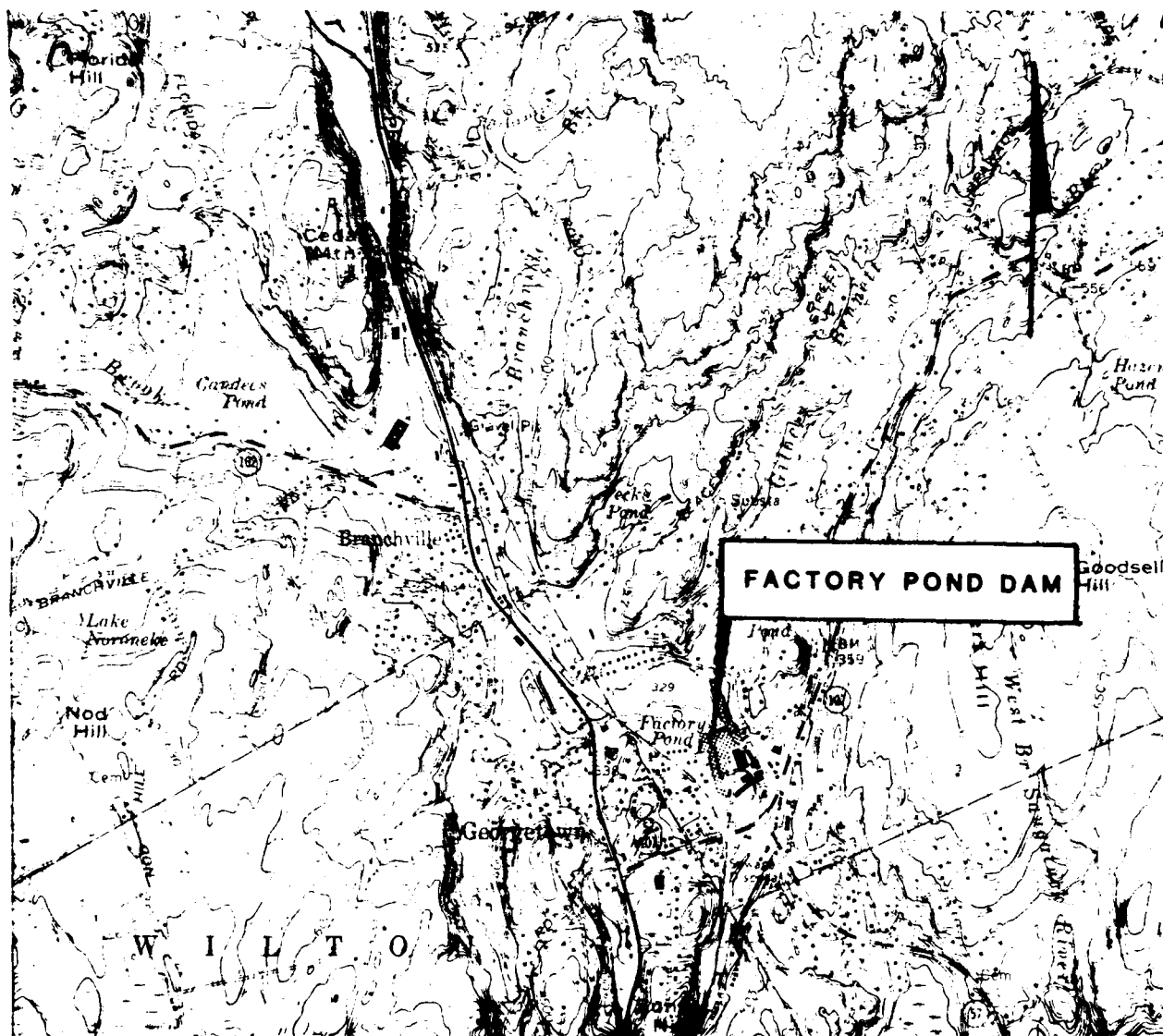
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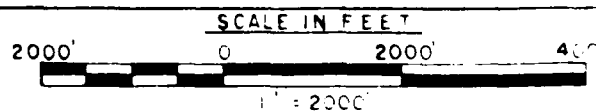


FACTORY POND DAM



QUADRANGLE **BETHEL, CT**

US ARMY, CORPS OF ENGINEERS
NEW ENGLAND DIVISION
WALTHAM, MASS.



LOCATION MAP

PHASE I INSPECTION REPORT
FACTORY POND DAM CT 00271

SECTION 1 - PROJECT INFORMATION

1.1 General

a. Authority - Public Law 92-367, August 8, 1972 authorized the Secretary of the Army, through the Corps of Engineers, to initiate a National Program of Dam Inspections throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Storch Engineers has been retained by the New England Division to inspect and report on selected dams in the State of Connecticut. Authorization and notice to proceed were issued to Storch Engineers under a letter of March 6, 1980 from William E. Hodgson, Jr., Colonel, Corps of Engineers. Contract No. DACW33-80-C-0035 has been assigned by the Corps of Engineers for this work.

b. Purpose of Inspection -

(1) Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.

(2) Encourage and prepare the states to initiate quickly effective dam safety programs for non-Federal dams.

(3) To update, verify and complete the National Inventory of Dams.

1.2 Description of Project

a. Location - Factory Pond Dam is located approximately 1,200 feet north of the Route 57 and Route 107 interchange and east of Route 7 in the Georgetown

section of the Town of Redding, Connecticut (See Location Map). The coordinates of the dam are approximately 41°-15.5' north latitude and 73°-26' west longitude. The dam is located on the Norwalk River in the Norwalk River Basin.

b. Description of Dam and Appurtenances - Factory Pond Dam is a combination of sheet piling/concrete fill and masonry that is 175 feet long and 18.75 feet high. The sheet piling portion of the dam consists of two rows of sheet piling spaced 5 feet apart, with concrete fill in between. The sheet piling extends to 20 feet below the top of the dam.

The spillway is a stone masonry weir with an ogee section that is 75 feet long. The top of the dam is 4.65 feet above the spillway crest. The spillway is located on the southern portion of the dam adjacent to a factory building. There is a 10-foot downstream apron with the remainder of the channel being riprap.

There is a 5-foot diameter discharge conduit that is used to lower the pond for repairs to the dam. This conduit has a variable pitch blade turbine in it that was once used for water power. Presently, the turbine is not used for power but the blades are used as a control valve. Control of the blades is from inside the factory.

c. Size Classification - Factory Pond Dam has a maximum height of 18.75 feet and a maximum storage of 192 acre-feet at the top of the dam. In accordance with the Recommended Guidelines for Safety Inspection of Dams established by the Corps of Engineers, the dam is classified as small (height less than 40 feet and storage less than 1,000 acre-feet).

d. Hazard Classification - Factory Pond Dam is classified as having a high hazard potential. Failure of the dam could result in the loss of more than a few lives and cause significant property damage. Approximately 50 feet

downstream is a Gilbert & Bennett manufacturing building spanning the river (See Appendix C - Photos 3 and 4). The floor of the building is approximately 8 feet above the streambed. Estimated flow and water depths just prior to dam failure at this location is 2,500 cfs at 8.5 feet and just after dam failure is 4,095 cfs at 15 feet. These depths are headwater depths for the twin 5'x24' openings under the building.

e. Ownership - The Factory Pond Dam is owned by:

Gilbert & Bennett
Georgetown, Connecticut

f. Operator - The person in charge of day-to-day operation of the dam is:

Mr. Dom Curtis
Gilbert & Bennett
Georgetown, Connecticut
(203) 544-8323

g. Purpose of Dam - The dam impounds the Factory Pond which serves as a primary water supply for industrial use by Gilbert & Bennett.

h. Design and Construction History - There are no design computations or drawings available for the original dam. During the Flood of "55", the dam was damaged as a result of water flowing through a low spot in the area of the west abutment. The water never overtopped the dam. The dam was reconstructed in 1956. Drawings are available for this reconstruction. Essentially, this reconstruction was of the western abutment, which is now sheet piling/concrete fill. The design was done by Industrial Associates, Inc., Philadelphia, Pennsylvania. In 1968, the masonry face was gunited.

i. Normal Operational Procedure - There is a regular maintenance staff at the plant that takes care of the dam. The water level of the pond is lowered if a major storm is imminent.

1.3 Pertinent Data

a. Drainage Area - Factory Pond drainage basin is in the Towns of Ridgefield, Redding, Wilton and Weston and is irregular in shape. The area of the drainage basin is 12.2 square miles (Appendix D - Plate 3). Approximately 10 percent of the drainage basin is natural storage and more than 80 percent is undeveloped. The topography is rolling with elevations ranging from 840 (NGVD) to 329 (NGVD) at the spillway crest.

b. Discharge at Damsite - There are no records available for discharge at the dam.

| | |
|--|-----------|
| (1) Outlet works (conduit) size: | 60 inches |
| Invert elevation (feet above NGVD): | 319 |
| Discharge Capacity at top of dam: | 40 cfs |
| (2) Maximum known flood at damsite: | 4,800 cfs |
| (3) Ungated spillway capacity at top of dam: | 2,500 cfs |
| Elevation (NGVD): | 333.65 |
| (4) Ungated spillway capacity at test flood elevation: | 6,700 cfs |
| Elevation (NGVD): | 337.9 |
| (5) Gated spillway capacity at normal pool elevation: | N/A |
| Elevation (NGVD): | N/A |
| (6) Gated spillway capacity at test flood elevation: | N/A |
| Elevation: | N/A |
| (7) Total spillway capacity at test flood elevation: | 6,700 cfs |

| | | |
|-----|--|-----------|
| | Elevation | 337.9 |
| (8) | Total project discharge at top of dam: | 2,540 cfs |
| | Elevation (NGVD): | 333.65 |
| (9) | Total project discharge at test flood elevation: | 8,250 cfs |
| | Elevation (NGVD): | 337.9 |
| c. | Elevation (feet above NGVD) | |
| (1) | Streambed at toe of dam: | 314.9 |
| (2) | Bottom of cutoff: | 313.65 |
| (3) | Maximum tailwater: | 323.4 |
| (4) | Normal pool: | 329 |
| (5) | Full flood control pool: | N/A |
| (6) | Spillway crest (ungated): | 329 |
| (7) | Design surcharge (original design): | unknown |
| (8) | Top of dam: | 333.65 |
| (9) | Test flood surcharge: | 337.9 |
| d. | Reservoir (length in feet) | |
| (1) | Normal pool: | 1,700 |
| (2) | Flood control pool: | N/A |
| (3) | Spillway crest pool: | 1,700 |
| (4) | Top of dam: | 1,800 |
| (5) | Test flood pool: | 2,000 |
| e. | Storage (acre-feet) | |
| (1) | Normal pool: | 100 |
| (2) | Flood control pool: | N/A |
| (3) | Spillway crest pool: | 100 |

| | | |
|----|---------------------------------|---|
| | (4) Top of dam: | 192 |
| | (5) Test flood pool: | 282 |
| f. | Reservoir Surface (acres) | |
| | (1) Normal pool: | 16.5 |
| | (2) Flood control pool: | N/A |
| | (3) Spillway crest: | 16.5 |
| | (4) Test flood pool: | 23 |
| | (5) Top of dam: | 20 |
| g. | Dam | |
| | (1) Type: | sheet piling/concrete fill & stone masonry |
| | (2) Length: | 175 |
| | (3) Height: | 18.75 |
| | (4) Top width: | 5 feet |
| | (5) Side slopes: | vertical |
| | (6) Zoning: | unknown |
| | (7) Impervious core: | N/A |
| | (8) Cutoff: | sheet piling down to elevation 313.65 (NGVD) |
| | (9) Grout curtain: | unknown |
| | (10) Other: | N/A |
| h. | Diversion and Regulating Tunnel | N/A |
| i. | Spillway | |
| | (1) Type: | stone masonry weir/ogee |
| | (2) Length of weir: | 75 feet |

- | | |
|---|---|
| (3) Crest elevation (without flashboard): | 329 |
| (4) Gates: | N/A |
| (5) U/S channel: | no channel-natural pond bottom |
| (6) D/S channel: | concrete apron and riprapped channel |
| (7) General: | N/A |
- j. Regulating Outlets
- | | |
|------------------------------|---|
| (1) Invert elevation (NGVD): | 319 |
| (2) Size: | 60 inches |
| (3) Description: | cast iron pipe |
| (4) Control Mechanism | manually operated gate |
| (5) Other: | valve is the variable pitch blades of the turbine |

SECTION 2 - ENGINEERING DATA

2.1 Design Data

There are no design computations or drawings for the original dam. However, there are drawings for the reconstructed portion of the dam that was damaged during the Flood of October, 1955. These drawings were prepared by Industrial Associates, Inc. of Philadelphia, Pennsylvania (See Appendix B).

2.2 Construction Data

No records are available for the original construction or the reconstruction. Drawings for the reconstruction are available (Appendix B).

2.3 Operation Data

The gate for the 5-foot diameter discharge conduit is operable and it is exercised periodically to lower the pond. Also, when the threat of a major storm is imminent, the pond is lowered.

2.4 Evaluation of Data

a. Availability - There were no computations available, however, there are some drawings available. These drawings are available from the Department of Environmental Protection (DEP).

b. Adequacy - The information made available along with the visual inspection, past performance history and hydraulic/hydrologic assumptions were adequate to assess the condition of the facility.

c. Validity - Due to the lack of available data, the conclusions and recommendations found in this report are based on the visual inspection and hydraulic/hydrologic computations.

SECTION 3 - VISUAL INSPECTION

3.1 Findings

a. General - The visual inspection was conducted on April 23, 1980 by members of the engineering staff of Storch Engineers, D. Baugh and Associates, Inc. and Matthews Associates with the help of Mr. Peter Harco and Dom Curtis of Gilbert & Bennett. A copy of the visual inspection check list is contained in Appendix A of this report. Selected photos of the dam and appurtenant structures are contained in Appendix C.

In general, the overall appearance and condition of the facility and its appurtenant structures is fair.

b. Dam - The dam is a combination of sheet piling/concrete fill and masonry. The sheet piling and concrete fill are in good condition (Photo 1). The sheet piling is painted with some areas that are rusting. The concrete cap is in good condition with no cracking (Photo 3). There is no evidence of settlement or lateral movement. There are some areas along the toe where vegetation is growing (Photo 1). The masonry portion of the dam is fair with some joints in need of repointing (Photo 8).

c. Appurtenant Structures - The inlet to the discharge conduit is protected by a bar screen which is in good condition. The discharge conduit itself was underwater and could not be inspected (Photo 8). The conduit contains a power turbine which is not used. The control for this conduit is by varying the pitch on the blades of the turbine. The gate is operable and the conduit was in use at the time of inspection.

The spillway is a fixed weir that appears to be in fair condition (Photos 2 and 5). The downstream training wall and western abutment (Photos 1 and 6) show some seepage and cracks.

d. Reservoir Area - The area immediately adjacent to the facility is gently sloped and in a natural state. The shoreline shows no signs of sloughing or erosion. There is some development adjacent to the reservoir, which is in the form of warehouses owned by Gilbert & Bennett. A rapid rise in the water level of the reservoir will not endanger any life or property.

e. Downstream Channel - The channel from the spillway is confined by buildings and many bridges (Photos 3, 4 and 7). It is a stone lined, but its capacity is questionable. Immediately downstream, the channel passes under a building (Photo 4). Under a large flow, the pier shown in the picture and the building may be destroyed.

3.2 Evaluation

Overall, the general condition of the dam is fair. The visual inspection revealed items that lead to this assessment, and apparent areas of distress such as:

- a. Seepage through the abutment.
- b. Need for repointing of the masonry.
- c. Vegetation along the toe of the dam.

SECTION 4 - OPERATIONAL AND MAINTENANCE PROCEDURES

4.1 Operational Procedures

a. General - The operation of this facility is strictly for the purpose of industrial use, and the water level is kept as full as possible. Water for industrial use is pumped out. The pond is lowered once a year when manufacturing operations are shut down.

b. Description of Any Warning System in Effect - The only formal operating procedure is when there is a threat of a substantial storm. When this occurs, the gate to the 5-foot diameter conduit is opened and the water level in the pond is lowered (5 feet in 24 hours). There is no system for warning downstream inhabitants.

4.2 Maintenance Procedures

a. General - The pond is drained each year during the manufacturing shut down. At this time, the mortar is repaired and the cracks are filled.

b. Operating Facilities - The gate to the 5-foot discharge conduit was taken apart and refurbished approximately ten years ago.

4.3 Evaluation

The dam is maintained on an annual basis. Although they do lower the pond prior to a major storm, there should be a formal warning system for downstream flooding.

SECTION 5 - EVALUATION OF HYDRAULIC/HYDROLOGIC FEATURES

5.1 General

Factory Pond Dam is a sheet piling/concrete fill and masonry dam approximately 175 feet long and 18.75 feet high. The spillway is a masonry weir, 75 feet long. The 5-foot diameter conduit is used to lower the pond before a major storm and according to maintenance personnel, it takes 24 hours to lower the pond 5 feet (14.2 acres x 5.0 feet - 24 hours = 35.8 cfs). Compared to the test flood, this flow is small. Therefore, this conduit is not in the hydrologic analysis.

The watershed encompasses 12.2 square miles and is 80 percent undeveloped. The topography is rolling with the terrain rising 511 feet from the spillway crest.

The pond has a total capacity of 192 acre-feet when the pond is at the top of the embankment and 100 acre-feet at the spillway crest. Therefore, there is approximately 92 acre-feet of storage available. The test flood outflow for this dam is 8,250 cfs and the spillway capacity is 2,500 cfs or approximately 30% of the test flood outflow.

5.2 Design Data

No design data is available.

5.3 Experience Data

Factory Pond Dam has experienced all the major storms of the 1930's and 1950's and most recently January, 1979. The flood of record resulted from the storm of October, 1955. The discharge at the site was approximately 4,800 cfs and the western portion of the dam was damaged, resulting in its reconstruction.

5.4 Test Flood Analysis

Based on the guidelines found in the Recommended Guidelines for Safety Inspection of Dams, the dam is classified as a small structure with a high hazard potential. The test flood for these conditions ranges from 1/2 the Probable Maximum Flood (PMF) to the PMF. One half the PMF was used for this dam because of the small size.

Using the guide curves established by the Corps of Engineers (rolling terrain), the test flood inflow is 9,640 cfs. The routing procedure established by the Corps gives an approximate outflow of 8,250 cfs. The spillway capacity is approximately 2,500 cfs or approximately 30% of the test flood outflow. The test flood will overtop the dam by approximately 4.3 feet. The building over the spillway (Photos 1 and 2) will not affect the test flood outflow.

Storage behind the dam was assumed to begin at the spillway crest. Storage was determined by an average area depth analysis. Capacity curves for the spillway assumed weir flow.

5.5 Dam Failure Analysis

A dam failure analysis was performed using the Rule of Thumb method in accordance with guidelines established by the Corps of Engineers. Failure was assumed to occur when the water level in the reservoir was at the top of the dam.

The spillway discharge just prior to dam failure is 2,500 cfs and will produce a depth of flow of approximately 8.5 feet immediately downstream (at Gilbert & Bennett's building over the channel) from the dam. The calculated dam failure discharge is 4,095 cfs and will produce a depth of flow of

approximately 15 feet immediately downstream from the dam. These depths are headwater depths for the twin 5'x24' openings under the building. The failure analysis covered a distance of approximately 1,000 feet downstream where the depth of flow was calculated to be 6.5 feet.

Failure of Factory Pond Dam may result in the loss of more than a few lives and the flood wave will destroy portions of the Gilbert & Bennett Factory which spans over the river 50 feet downstream. The floor of the building is approximately 8.0 feet above the streambed, therefore, at failure, water depths in the building will be approximately 7 feet. Also at least two dwellings located approximately 1,500 feet downstream will sustain some damage. Water depths in these dwellings will be approximately 2 feet.

SECTION 6 - EVALUATION OF STRUCTURAL STABILITY

6.1 Visual Observations

The general structural stability of the dam is good as evidenced by the vertical, horizontal and lateral alignment. The mortared stone spillway is in fair condition with some cracks in the concrete at the westerly end. The steel sheet piling is in good condition.

The only area of concern is at the western abutment/training wall where there is some cracking and some seepage. This seepage, at the time of inspection, was negligible.

6.2 Design and Construction Data

The original design and construction data are not available. There are construction drawings available for the reconstruction of the dam.

6.3 Post-Construction Changes

Since the reconstruction of the dam, the only changes, except for minor maintenance work, are the guniting of the stone masonry face of the spillway in 1968 and the addition of the covered passageway over the dam (Appendix B - Plate 1). One of the piers for this passageway was constructed across the outlet channel. This outlet channel has since ceased to function.

6.4 Seismic Stability

The dam is located in Seismic Zone 1 and in accordance with Recommended Phase I Guidelines does not warrant a seismic analysis.

SECTION 7 - ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

7.1 Dam Assessment

a. Condition - After consideration of the available information, the results of the inspection, contact with the owner and hydraulic/hydrologic computations, the general condition of the Factory Pond Dam is fair.

b. Adequacy of Information - The information available is such that an assessment of the safety of the dam should be based on the available data, the visual inspection results, past operational performance of the dam and its appurtenant structures and computations developed for this report.

c. Urgency - It is considered that the recommendations suggested below be implemented within one year after receipt of this Phase I Inspection Report.

7.2 Recommendations

The following recommendations should be carried out under the direction of a qualified registered engineer.

- a. Seepage through the spillway abutment should be investigated further to determine its origin and monitored to determine any changes.
- b. Prepare a detailed hydraulic/hydrologic study to determine spillway adequacy and an increase of the total project discharge if necessary.

7.3 Remedial Measures

- a. Operation and Maintenance Procedures -

(1) Repair all cracks and mortar all joints in the masonry portion of the dam.

(2) Vegetation along the toe of the dam should be removed. This will facilitate the visual observation of existing and potential seepage.

(3) Plans for around-the-clock surveillance should be developed for periods of unusually heavy rains and a formal warning system should be put into operation for use in the event of an emergency.

(4) A program of annual technical inspection should be established.

7.4 Alternatives

None.

Information pertaining to the history, maintenance and modification to
Factory Pond Dam as well as copies of past reports are located at:

State of Connecticut
Department of Environmental Protection
Water Resources Unit
State Office Building
Hartford, Connecticut 06115

APPENDIX A

INSPECTION CHECK LIST

INSPECTION CHECK LIST

PARTY ORGANIZATION

PROJECT FACTORY POND DAM

DATE 4/23/80

TIME 9:00 a.m.

WEATHER Clear

W.S. ELEV. _____ U.S. _____ DN.S. _____

PARTY:

- | | |
|---|---|
| 1. <u>John F. Schearer , SE Civil</u> | 6. <u>Peter Haroo Gilbert & Bennett</u> |
| 2. <u>John Pozzato , MA Mech.</u> | 7. _____ |
| 3. <u>Kenneth J. Pudeler, SE Civil</u> | 8. _____ |
| 4. <u>Michael Haire , DBA Struct/Geo.</u> | 9. _____ |
| 5. <u>Peter Austin , DBA Civil</u> | 10. _____ |

| PROJECT FEATURE | INSPECTED BY | REMARKS |
|-----------------|--------------|---------|
| 1. _____ | | |
| 2. _____ | | |
| 3. _____ | | |
| 4. _____ | | |
| 5. _____ | | |
| 6. _____ | | |
| 7. _____ | | |
| 8. _____ | | |
| 9. _____ | | |
| 10. _____ | | |

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

| AREA EVALUATED | CONDITIONS |
|---|--|
| <u>DAM EMBANKMENT</u> | |
| Crest Elevation | Fair |
| Current Pool Elevation | Fair |
| Maximum Impoundment to Date | Never overtopped |
| Surface Cracks | Few (minor) |
| Pavement Condition | N/A |
| Movement or Settlement of Crest | None observed |
| Lateral Movement | Overall-good; |
| Vertical Alignment | Good |
| Horizontal Alignment | Good |
| Condition at Abutment and at Concrete Structures | Cracked joints noted in spillway side abutment |
| Indications of Movement of Structural Items on Slopes | None |
| Trespassing on Slopes | Not allowed |
| Vegetation on Slopes | None |
| Sloughing or Erosion of Slopes or Abutments | None |
| Rock Slope Protection - Riprap Failures | N/A |
| Unusual Movement or Cracking at or near Toes | None |
| Unusual Embankment or Downstream Seepage | Negligible |
| Piping or Boils | None |
| Foundation Drainage Features | None |
| Toe Drains | None |
| Instrumentation System | None |

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

AREA EVALUATED

CONDITION

CUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE

a. Approach Channel

Slope Conditions

Good

Bottom Conditions

Good

Rock Slides or Falls

None

Log Boom

Good condition (spans pond)

Debris

Negligable (periodically cleaned out)

Condition of Concrete Lining

None observed

Drains or Weep Holes

None

b. Intake Structure

Condition of Concrete

Good

Stop Logs and Slots

Good

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

| AREA EVALUATED | CONDITION |
|--|--|
| <u>OUTLET WORKS - CONTROL TOWER</u> | |
| a. Concrete and Structural | |
| General Condition | |
| Condition of Joints | |
| Spalling | |
| Visible Reinforcing | |
| Rusting or Staining of Concrete | |
| Any Seepage or Efflorescence | |
| Joint Alignment | |
| Unusual Seepage or Leaks in Gate Chamber | |
| Cracks | |
| Rusting or Corrosion of Steel | |
| b. Mechanical and Electrical | |
| Air Vents | None |
| Float Wells | None |
| Crane Hoist | None |
| Elevator | None |
| Hydraulic System | None |
| Service Gates | 1-5' Penstock, Hand-operated worm gear - good condition. |
| Emergency Gates | Process water used by factory: valve pit & 10" hand valve - good condition |
| Lightning Protection System | None |
| Emergency Power System | None |
| Wiring and Lighting System in Gate Chamber | Within factory bldg., in good condition |

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

| AREA EVALUATED | CONDITION |
|--|--------------|
| <u>OUTLET WORKS - TRANSITION AND CONDUIT</u> | Inaccessible |
| General Condition of Concrete | " |
| Rust or Staining on Concrete | " |
| Spalling | " |
| Erosion or Cavitation | " |
| Cracking | " |
| Alignment of Monoliths | " |
| Alignment of Joints | " |
| Numbering of Monoliths | " |
| A-5 | |

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

AREA EVALUATED

CONDITION

OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL

General Condition of ~~Concrete~~ ^{Stone masonry} Fair

Rust or Staining N/A

Spalling None

Erosion or Cavitation None

Visible Reinforcing None

Any Seepage or Efflorescence None

Condition at Joints Fair

Drain holes Fair

Channel

Loose Rock or Trees Overhanging
Channel None

Condition of Discharge Channel Good

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM DATE 4/23/80
 PROJECT FEATURE _____ NAME _____
 DISCIPLINE _____ NAME _____

| AREA EVALUATED | CONDITION |
|--|--|
| <u>OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS</u> | |
| a. Approach Channel | |
| General Condition | Underwater |
| Loose Rock Overhanging Channel | None |
| Trees Overhanging Channel | None |
| Floor of Approach Channel | Silted, otherwise good |
| b. Weir and Training Walls | |
| General Condition of Concrete | Good, but mortared joints on westerly training wall and abutting weir conc. cracked. |
| Rust or Staining | None |
| Spelling | None |
| Any Visible Reinforcing | None |
| Any Seepage or Efflorescence | Minor - westerly training wall/abutment |
| Drain Holes | None |
| c. Discharge Channel | |
| General Condition | Fair |
| Loose Rock Overhanging Channel | None |
| Trees Overhanging Channel | None |
| Floor of Channel | Good |
| Other Obstructions | Several walkway bridges and buildings overhang the channel |

INSPECTION CHECK LIST

PROJECT FACTORY POND DAM

DATE 4/23/80

PROJECT FEATURE _____

NAME _____

DISCIPLINE _____

NAME _____

AREA EVALUATED

CONDITION

OUTLET WORKS - SERVICE BRIDGE

N/A

a. Super Structure

Bearings

Anchor Bolts

Bridge Seat

Longitudinal Members

Under Side of Deck

Secondary Bracing

Deck

Drainage System

Railings

Expansion Joints

Paint

b. Abutment & Piers

General Condition of Concrete

Alignment of Abutment

Approach to Bridge

Condition of Seat & Backwall

APPENDIX B

ENGINEERING DATA

NEW YORK LICENSE 4788
CONNECTICUT REGISTRATION 4

JOSEPH W. CONE
CIVIL ENGINEER
124 HAVEMEYER PLACE
GREENWICH, CONNECTICUT

| | |
|------------------------|-------|
| COMMISSION RECEIVED | |
| MAY 23 1963 | |
| ANSWERED | |
| REFERRED | |
| FILED | |

TELEPHONE
TOWNSEND 9-2152

May 21, 1963

Mr. Emitt A. Dell, Field Inspector
Water Resources Commission
State Office Building
Hartford 15, Conn.

Re: Dam #1 Norwalk River
Gilbert & Bennett, Mfg. Co.

Dear Mr. Dell:

As requested, I inspected the above captioned dam on May 8, 1963. Many material changes have been made since I last saw the dam on August 13, 1957, when I read the weir gauge at the leak and estimated total flow at about 400,000 g.p.d. with FL in pond down about 6'. On May 8, '63 flow appeared to be greater than on August 13, '57 and I estimated flow at about 500,000 g.p.d., including small flows near west abutment of spillway. Flow at main leak shows in photo #2 enclosed.

More important reference correspondence

- (1) Dec. 23 '58 Cone to Wise
- (2) Jan. 9 '59 Mulliken to Wise
- (3) Feb. 1 '61 " " "
- (4) Feb. 8 '61 Cone " " re (3)

The material changes are shown very approximately on the enclosed photo of a topo sheet and not to scale.

Nos. 1 & 2 - New buildings

No. 3 - Channel widening and under old buildings.

Do not know increased area. But this has no particular relation with safety of dam.

Several piers are in channel for future building construction.

No. 4 - Combined retaining and training wall; retaining to hold fill and presumably future building; training to direct flow lines from sluice gate.

No. 5 - Drive through gate in fence from upper level El. 104.5 $\frac{1}{2}$, datum spillway crest at 100.0, to lower level at building (2).

No. 6 - Leaks in sluice gate chute and at west end of dam to be corrected.

No. 7 - Suggest walls be raised as noted hereinafter.

No. 8 - Entire area has been raised several feet above El. 104.5 by material excavated for new building at (2).

No. 9 - Leaks at west end.

In addition: (a) Channel through grounds has been improved; (b) New bridge with greater waterway area at Route #465; (c) New twin box culvert added at new road Route #53.

Incidentally, I understand that new waterways at Route #465 and Route #53 will pass at least 2200 cfs with the usual clearance requirement of 2' between design flood water surface and underside of deck. The combination of

old and new box culverts at Route #53 under severe flood conditions can pass about 4000 cfs with H of 3' and Vm less than 10.

I would observe that design flood flow for passing floods through a valley is an entirely different matter than design flood flow for a dam; design for the one is influenced largely by the B-C ratio (benefit to cost); the design for a dam is imperatively concerned with safety. Leak. The leak at and near the sluiceway of about 500,000 g.p.d. could become a serious matter and cause failure of the westerly portion of the dam. Leaks must be controlled; therefore Recommendation No. 2.

A weir was installed in August 1957 to determine whether or not flow varied with changes in flow line of pond. The plant superintendent said he would have his plant carpenter note readings on the gauge and report to me. I instructed the carpenter how to measure from a mark on the gauge to water surface. No readings were furnished me. I did take one reading on Aug. 13, 1963.

Sheet Piling. Top of piling as shown on photo #1 averages over 1 foot below top of west masonry abutment and wall. I understand piles were 20' long and are tied to anchors and therefore are at minimum depth in original ground. Steel will deteriorate in about twenty years to

a condition requiring filling sluiceway with solid concrete to protect west end of main dam. Conditions must be checked periodically.

Conditions Now. Again referring to photo #1. This photo was taken from top of training wall at bend in same. The wall does not show in the left portion of the photo but it does reinforce toe of pavement to the extent that there is less likelihood of piling kicking out when dam is overtopped and scouring takes place. Also concrete, shown in photo to left of pipe, tends to reinforce paving.

To right of photo, but not shown, is the old projecting abutment wall. There are small leaks here and some stone in base moved. This area should be reinforced with massive concrete; therefore Recommendation No. 3.

Spillway. The old spillway could pass approximately 1100 cfs without serious overtopping. My estimate is that flood of October '55 was at least 4000 cfs at the dam. I was told there was only minor flow over east abutment in '53. Therefore the west end of dam must have failed at a flow of 1100 cfs or less or at less than one third of probable maximum flow.

Cone's letter (1) Dec. 23 '58 recommended among other matters --- "(3) Extend the overflow masonry dam to full width of the valley".

May 21, 1963

Mulliken's letter (3) Feb. 1 '61. "We recently developed a scheme to construct additional spillway capacity to the west of our present dam-----". And "As soon as a drawing has been completed, we will send you a copy for your preliminary study----". I have never seen such plan.

This extension of the spillway would have provided a total capacity of over 4200 cfs with present H of 4.6 and new length 130' effective ($Q = 3.4 \times 130 \times 4.6^{3/2}$ (9.8) = 4230 cfs). And with H = 6 - 6500 cfs.

But the new building, walls, drives, etc. at #2 have most effectively checkmated this proposed solution which was practical and safe. Consequently I suggest raising all abutment walls to provide H = 7 as shown in recommendations.

As for the proposed diversion canal and conduits, this proposal is also blocked. However I always considered this a pipe-dream and not to be considered seriously.

It is my opinion that if additional spillway capacity is not provided the westerly portion of the dam will again fail due to overtopping and consequential scouring during a major flood.

I would remark that whatever Q was in October 1955 a future storm, in about 25 years, identical in every characteristic with 1955 storm would produce a much greater Q even up to 25%, because of:- (a) More intensive land use; (b) New bridges and culverts with greater waterway, thereby reducing valley storage; (c) Encroachment on and filling low areas, thereby again reducing valley storage; (d) Draining low areas.

I do not presently recommend an order to remove the dam once and for all, economic and other implications are evident.

I do recommend an order to make the corrections enumerated below or others of equivalent performance.

- (1) Owner furnish a map of conditions as they now are, showing in plan and elevations complete information. Particular reference to building (1) and old buildings and walls at (7). I have never seen a plan of this dam.
- (2) Effectively stop leaks at (9) west end of dam by grouting or otherwise.
- (3) When pipe at leaks has been completed and leaks stopped, place huge block of concrete between training wall and end of sluiceway and against bottom of spillway west abutment to reinforce same.

Mr. Emmitt A. Dell

-7-

May 21, 1963

- (4) Raise walls at (7), to not less than El. 7.0 (west abutment and walls protecting old building) - this means raising about 2.5' minimum.
- (5) Uncover and measure upstream face of spillway to determine whether or not section is safe under extra H of 2.5'.
If not safe reinforce front of dam with concrete.

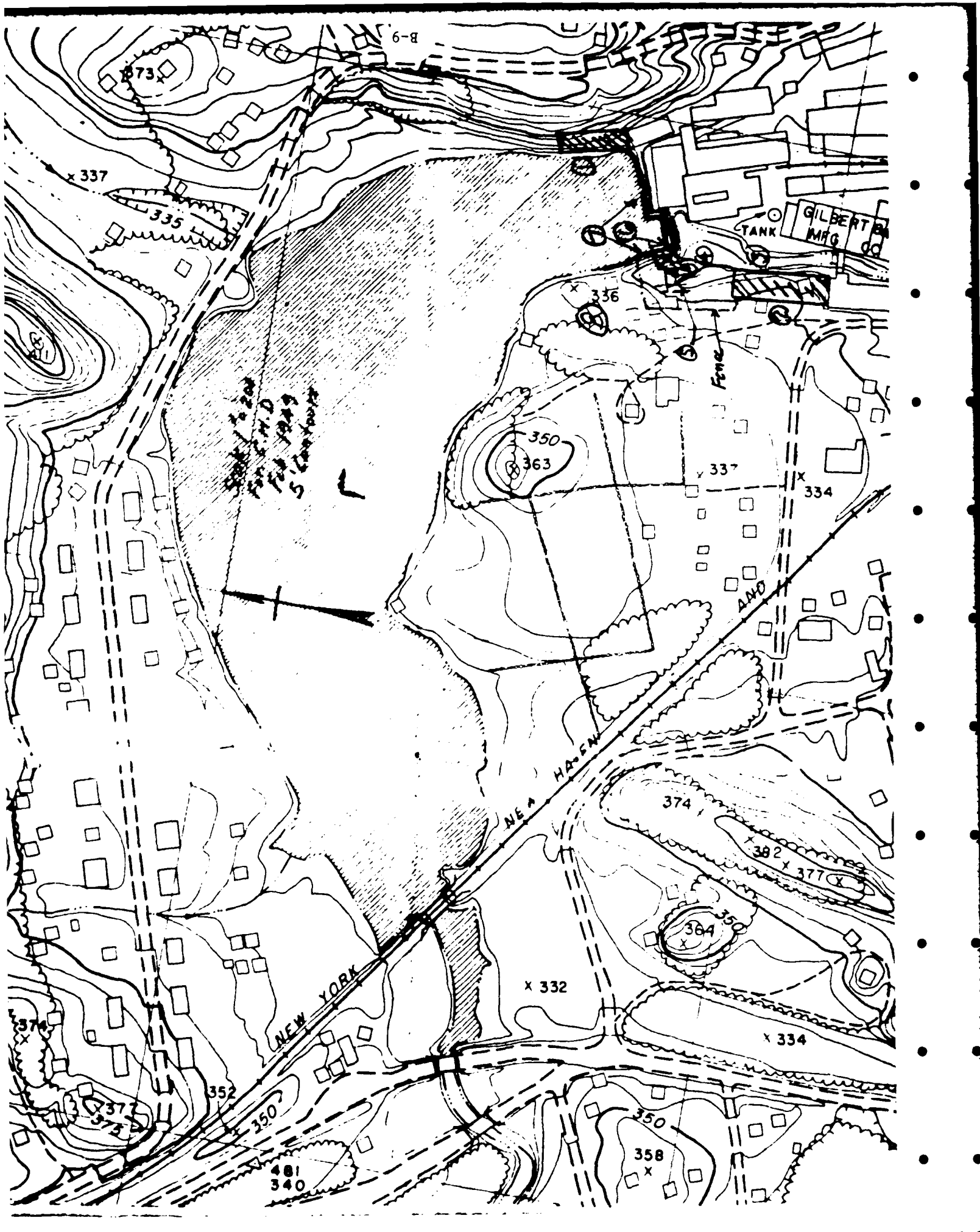
Purpose of the above is to pass a reasonable design flood without washing out the westerly portion, as occurred in 1955, resulting in material damage not only to Gilbert and Bennett but to other properties and highway structures downstream.

Enclosed are two photographs, photo of general data notes and print of revised Recurrence Curve for Conn. Formula.

Yours very truly,


G. W. Cone

JWC/dr
Enc: 2 photographs
2 prints



B-9

373

x 337

335

TANK

GILBERT
MFG CO

x 336

Fence
AWO
350
363
337
334

350

363

x 337

x 334

NEA HAN

374

x 382

x 377

364

NEW YORK

x 332

x 334

352

350

481
340

350

358
x

GEN. NOTES

5/20/63

Max Q without overtopping

Old Spilling 1050 cfs. with $H = 2.7$ Present " 2250 " " $H = 4.6$

'55 Flood. 3100 " on 8± sq. mi by USGS

" " \pm 400± " per sq mi on 8 sq mi

Watershed 12.1 sq. mi at dam

'55 Flood 4800± cfs. (400x12) at dam Rough Q prob. some too high for dam.

Proposed Spilling 4000 cfs with $H = 7$ minimum

MAF 850 cfs with CFS1 by Conc

Rough " check 810 " wtd. aver. of 8 sheet sheds

$$\left. \begin{array}{l} \text{Min. Design } R = \\ H = 7 \end{array} \right\} \frac{Q}{MAF} = \frac{4000 \text{ cfs}}{800} = 5 \pm 0.66\% \text{ chance (150 yr)}$$

$$'55 R \quad \frac{4800}{800} = 6 \pm 0.5\% \text{ " (200 yr)}$$

Total precip. 12" Danbury at 14-17 in 155

" 9.6 Derby "

" 13.8 Stamford "

Precip of 9" Prob max in 24 hr on 12.5 sq. mi. and approx R is .7% (150 yr)

But prob nearly max runoff because of antecedent rain with swamps and brooks full.

1" per hr on 12.5 sq. mi 7,680 cfs if 100% runoff.

D.C.D

RECORDS - 10/16/76
CIVIC 12/18/76

LOC.

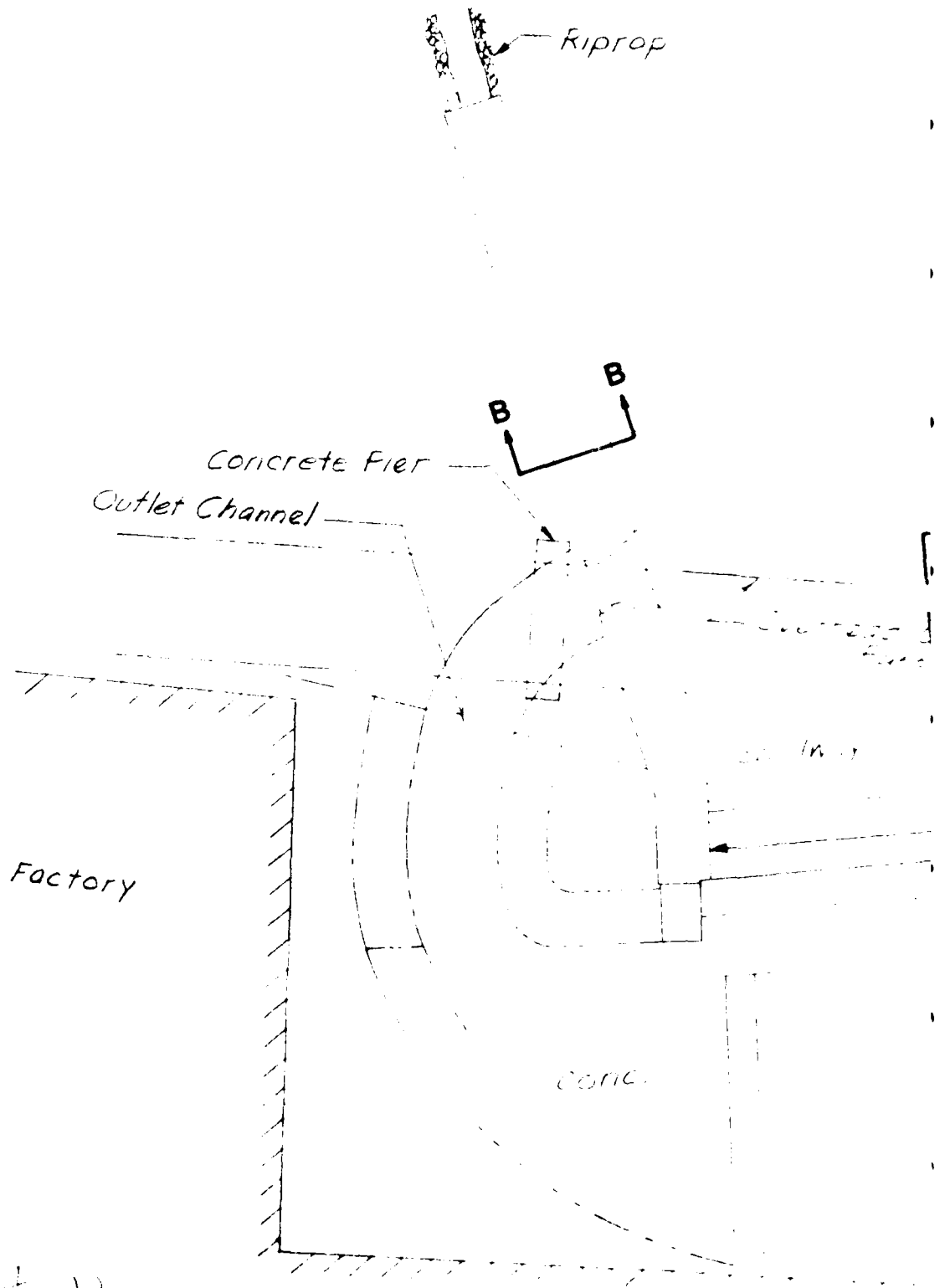


PLATE 1

Log Boom

Riprap

POND

Sur
1914

Overhead Covered
Passageway

L-shaped Cove
Passageway

Spillway

75'

Factory

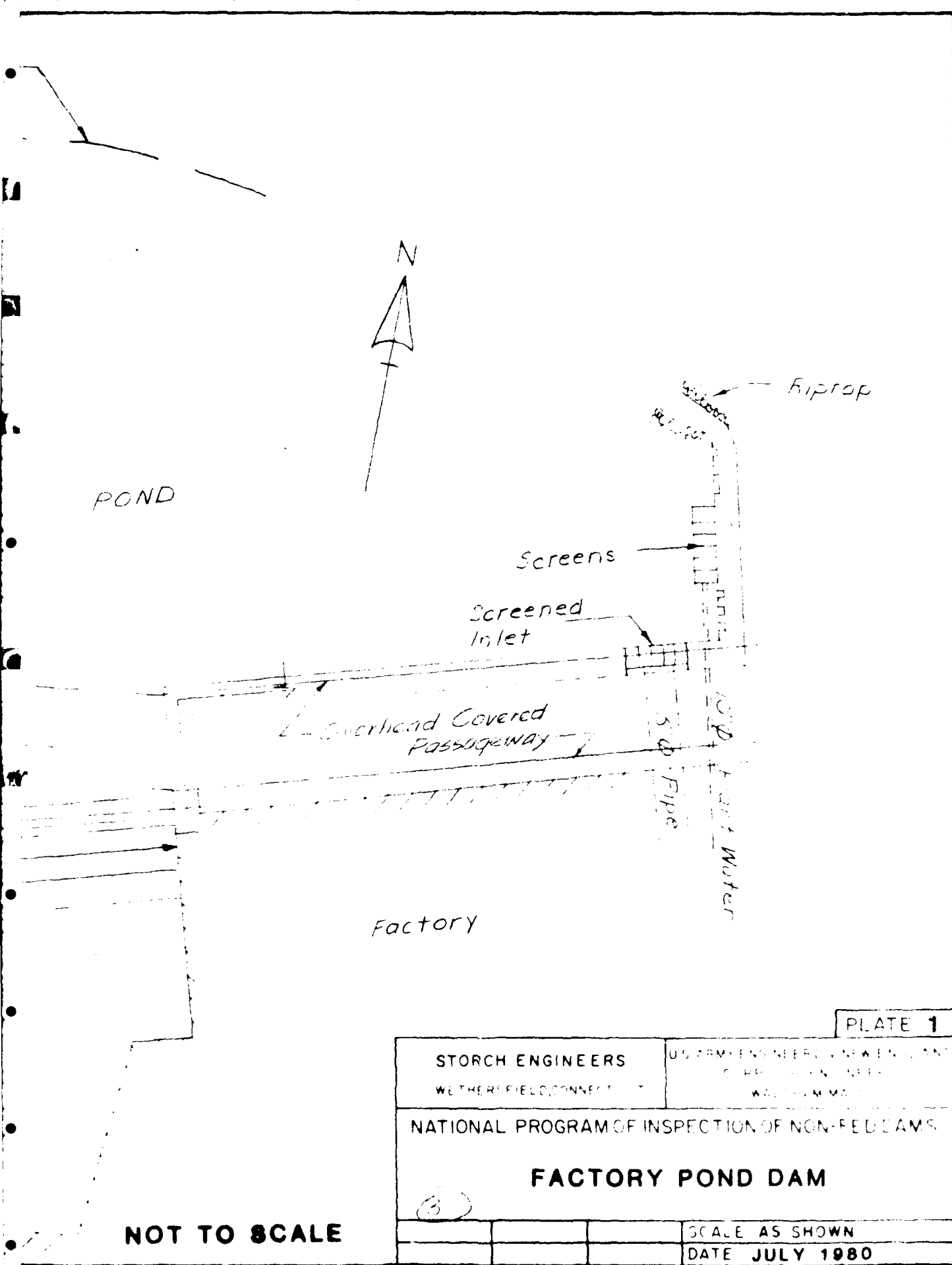
CORIC

STOR H

W. T. H. H.

NATIONAL

NOT TO SCALE



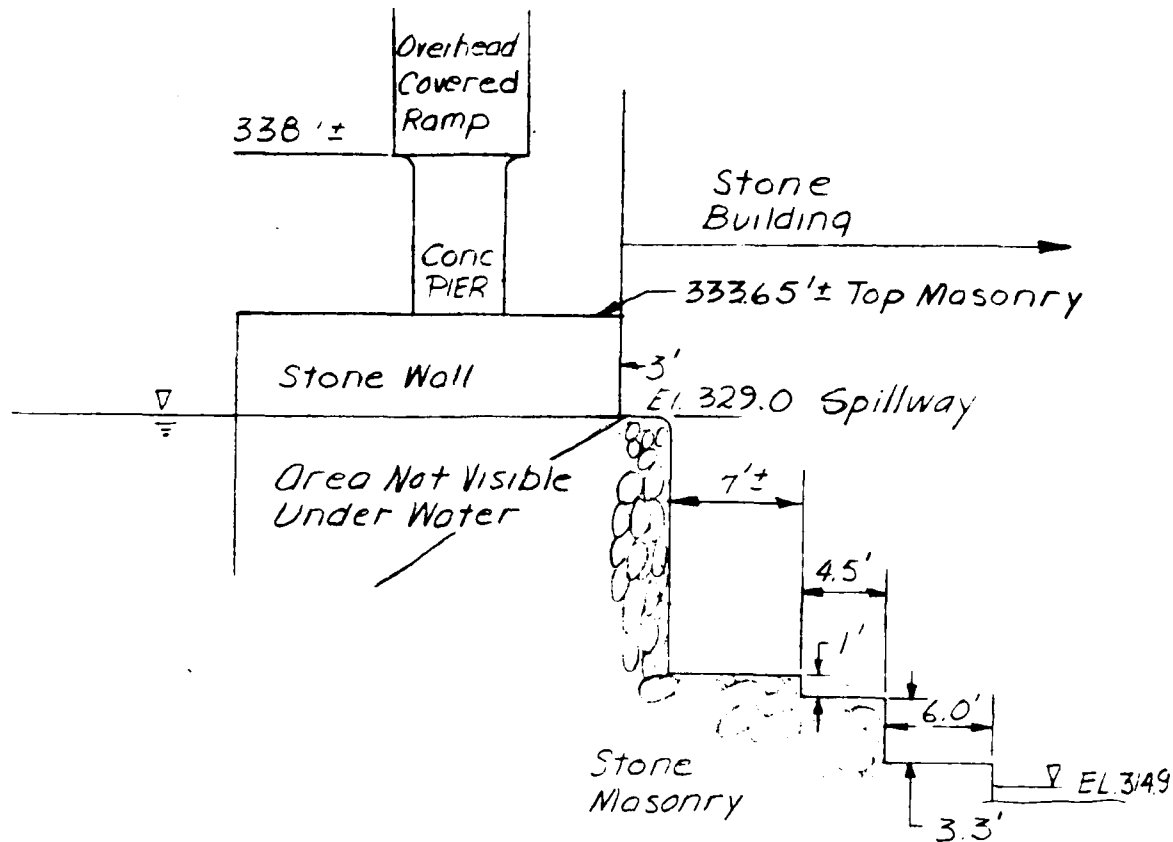
NOT TO SCALE

PLATE 1

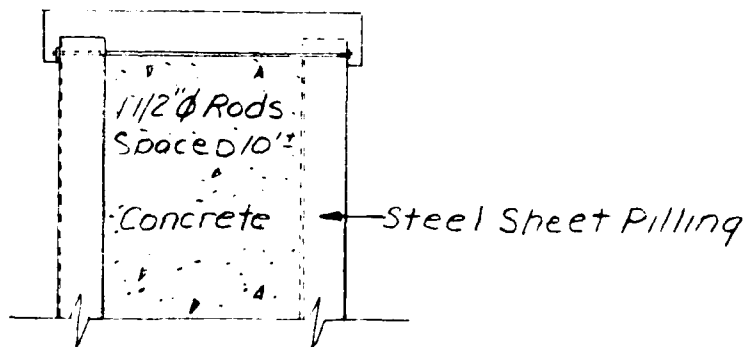
| | | | |
|--|--|--|--|
| STORCH ENGINEERS WETHERSFIELD, CONNECTICUT | | US ARMY ENGINEER CORPS WATERWAYS DIVISION WASHINGTON, D.C. | |
| NATIONAL PROGRAM OF INSPECTION OF NON-FED DAMS | | | |
| FACTORY POND DAM | | | |
| (3) | | SCALE AS SHOWN | |
| | | DATE JULY 1980 | |

APPENDIX C

PHOTOGRAPHS



SECTION A-A



SECTION B-B

PLATE 2

STORCH ENGINEERS
WETHERSFIELD, CONNECTICUT

U.S. ARMY ENGINEER DIV NEW ENGLAND
CORPS OF ENGINEERS
WALTHAM MASS

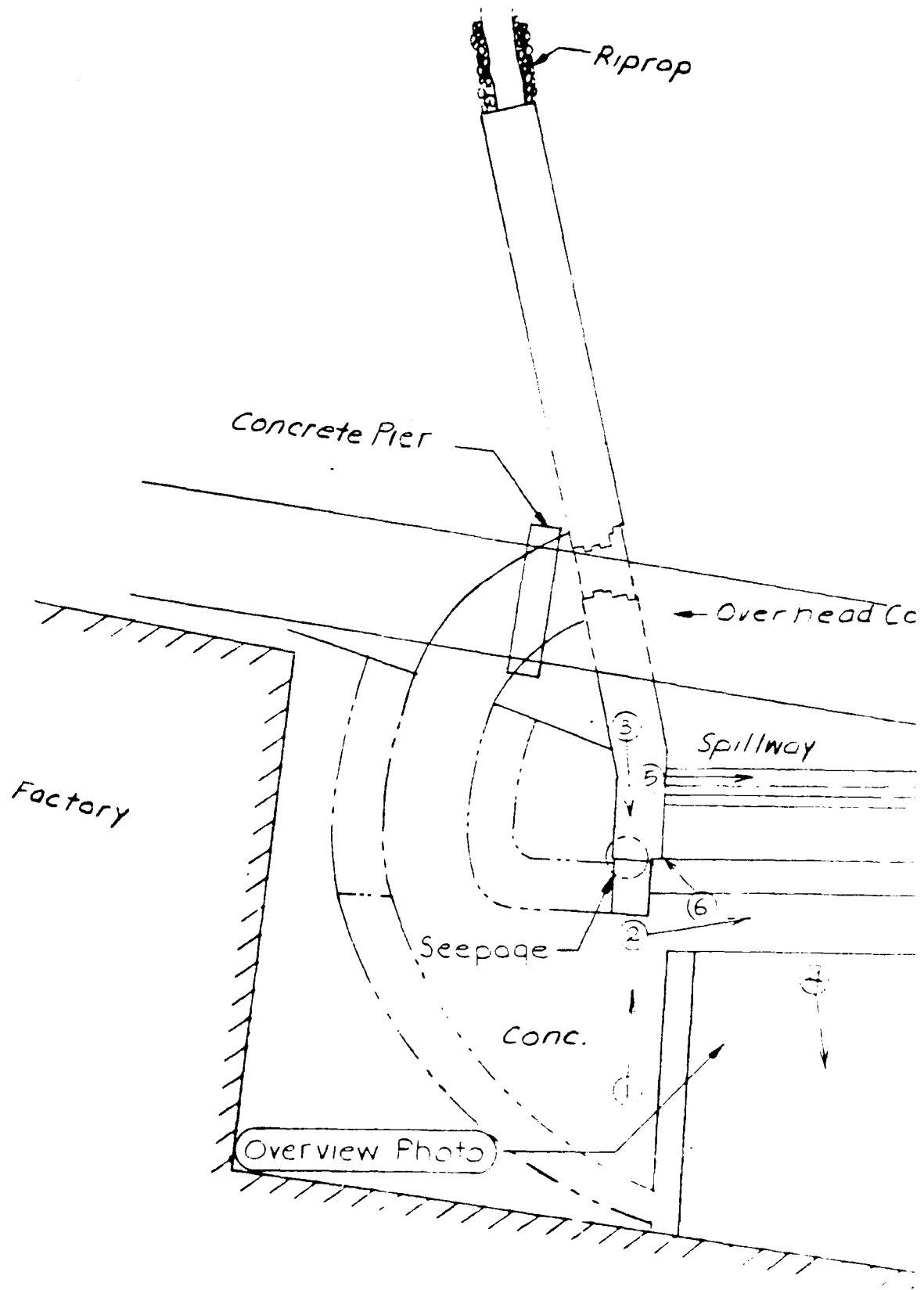
NATIONAL PROGRAM OF INSPECTION OF NON-FED. DAMS

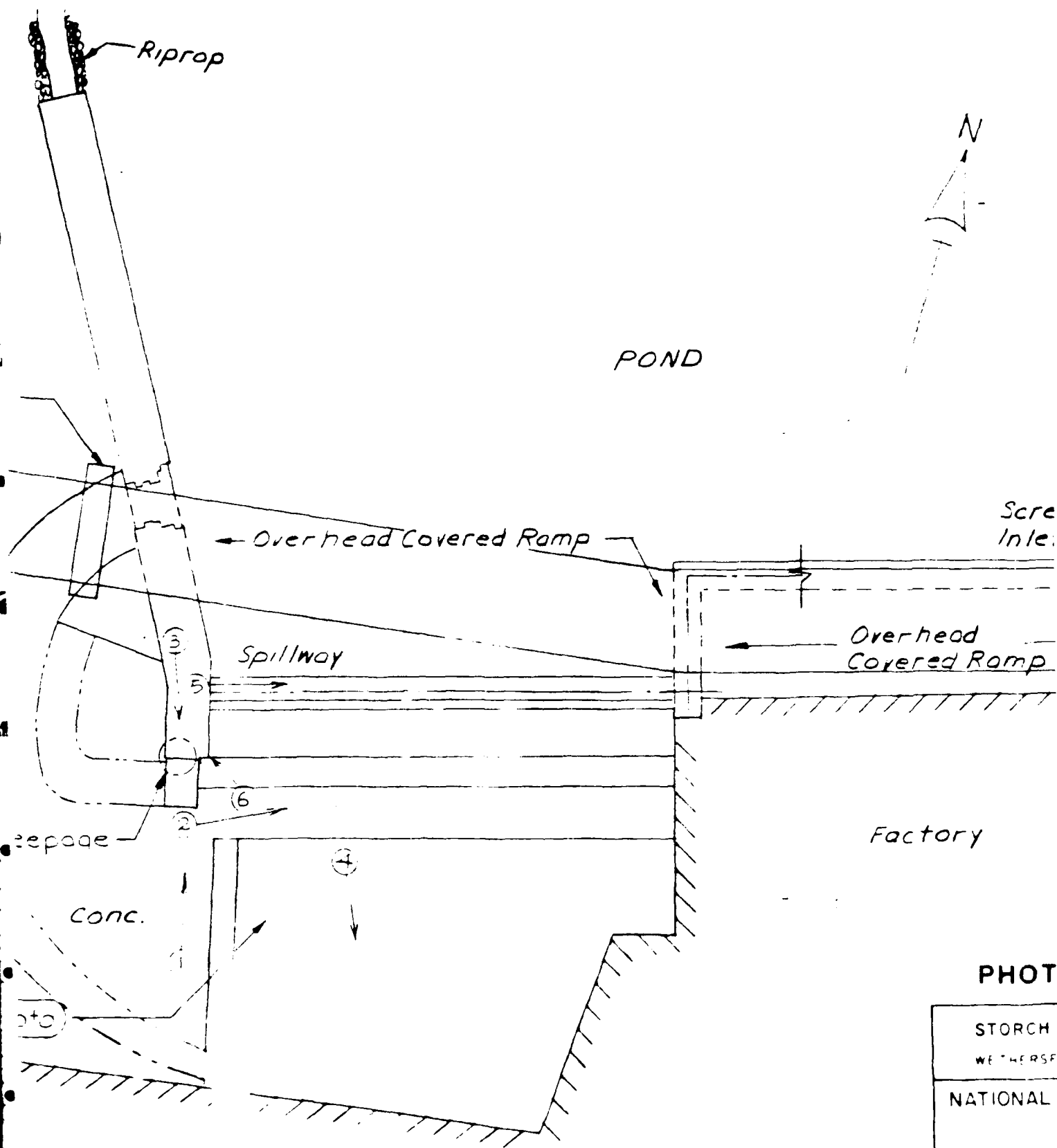
FACTORY POND DAM

NOT TO SCALE

SCALE AS SHOWN

DATE JULY 1980





PHOTO

| |
|----------|
| STORCH |
| WEATHERS |
| NATIONAL |
| 5 |

NOT TO SCALE

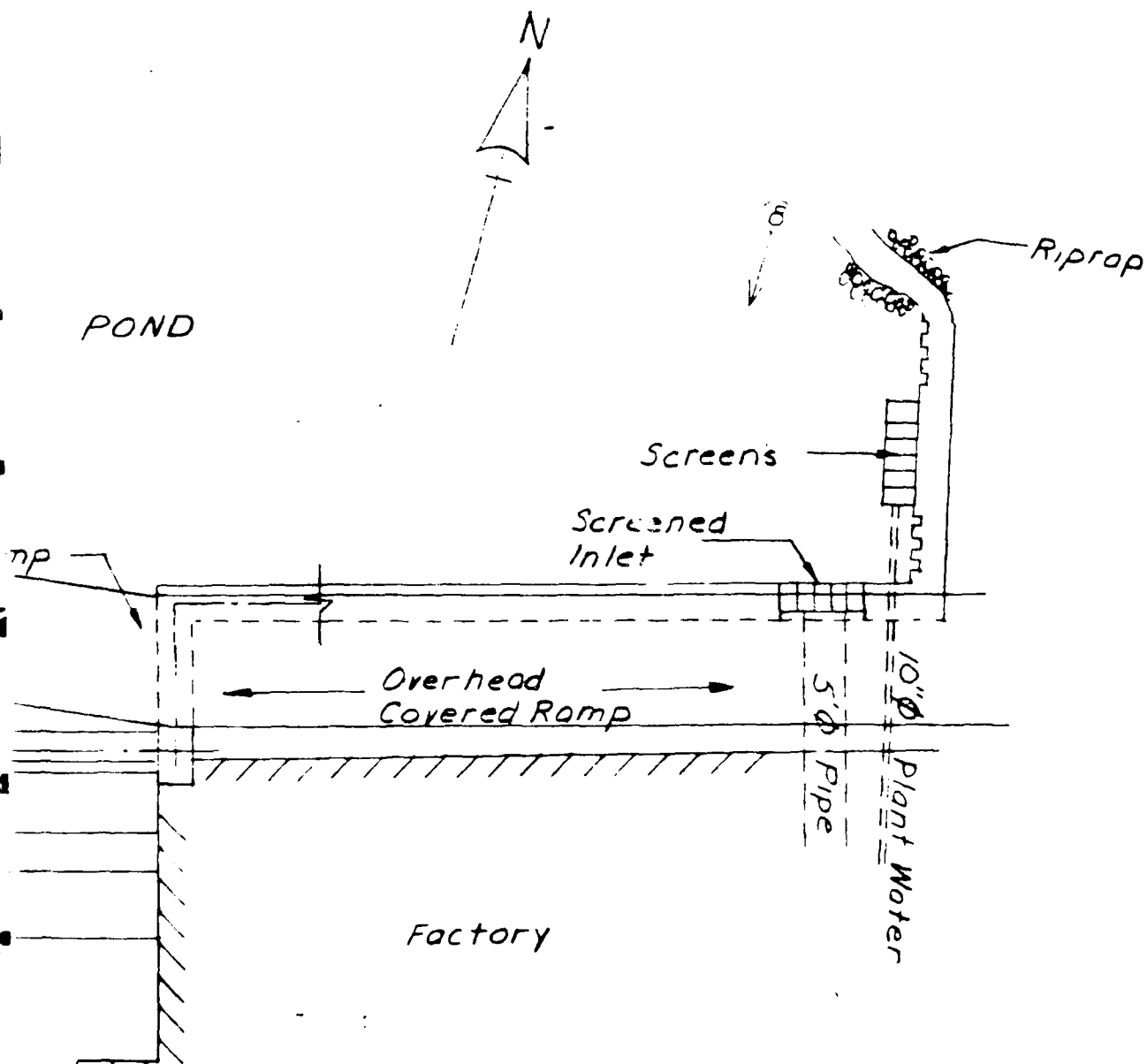


PHOTO LOCATION PLAN

PLATE 3

STORCH ENGINEERS
WETHERSFIELD, CONNECTICUT

US ARMY ENGINEER DIVISION NEW ENGLAND
CORPS OF ENGINEERS
WALTHAM MASS

NATIONAL PROGRAM OF INSPECTION OF NON-FED DAMS

FACTORY POND DAM

(3)

NOT TO SCALE

SCALE AS SHOWN

DATE JULY 1960

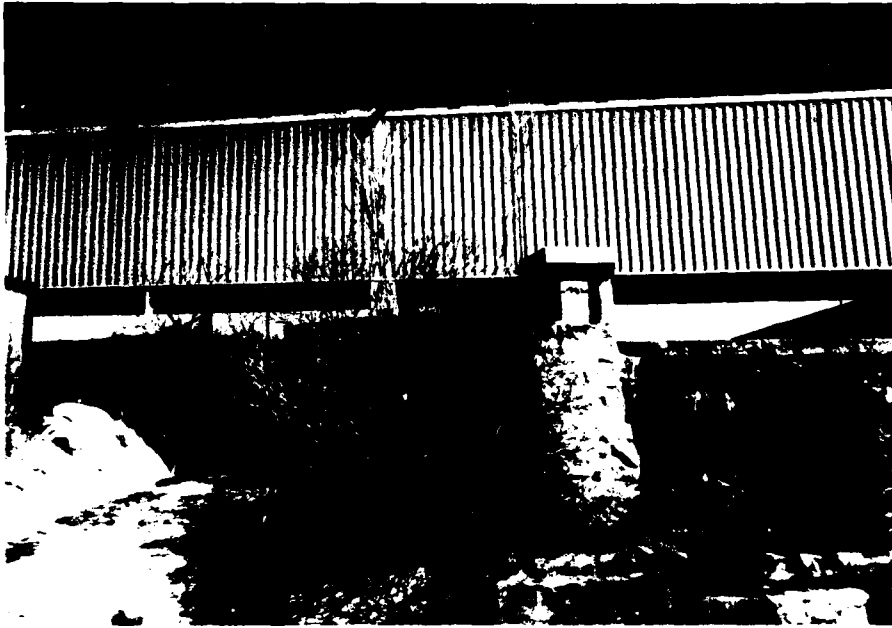


PHOTO 1
DOWNSTREAM FACE OF DAM



PHOTO 2
DOWNSTREAM FACE OF DAM



PHOTO 3
DOWNSTREAM CHANNEL



PHOTO 4
DOWNSTREAM CHANNEL

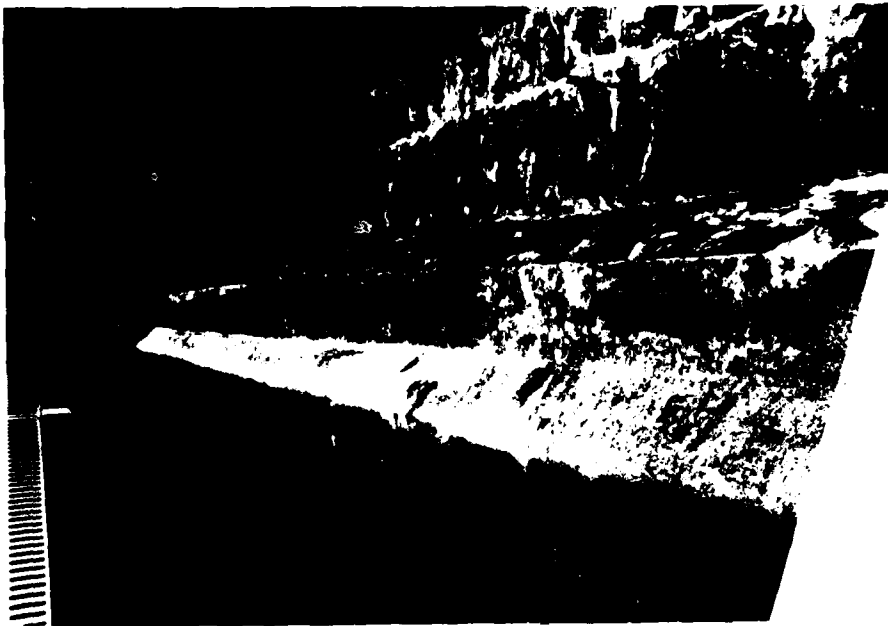


PHOTO 5
SPILLWAY CREST



PHOTO 6
SEEPAGE THROUGH WEST SPILLWAY ABUTMENT



PHOTO 7
DOWNSTREAM CHANNEL

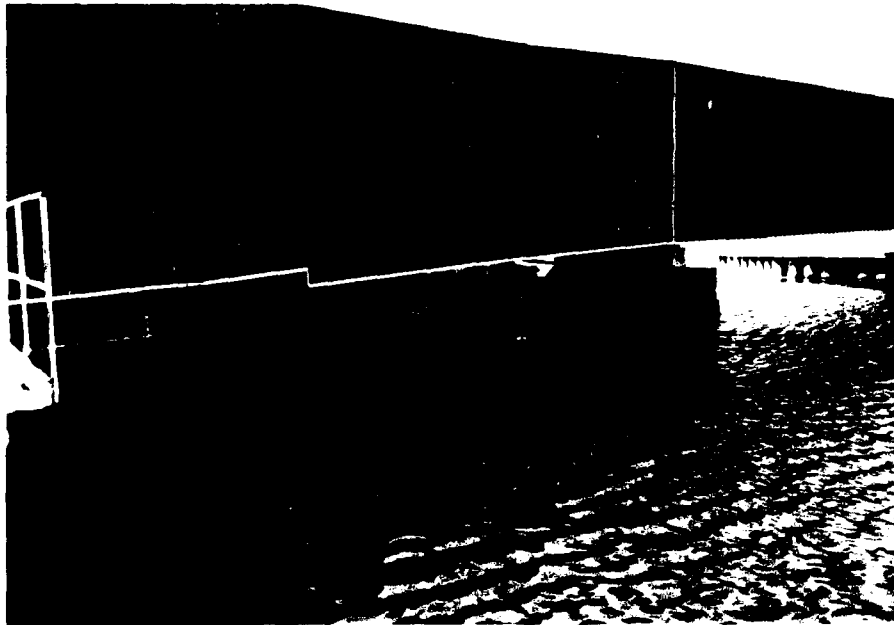


PHOTO 8
SCREEN & DIVERSION INTAKE
UPSTREAM FACE OF DAM
C-IV

APPENDIX D

HYDROLOGIC AND HYDRAULIC COMPUTATIONS

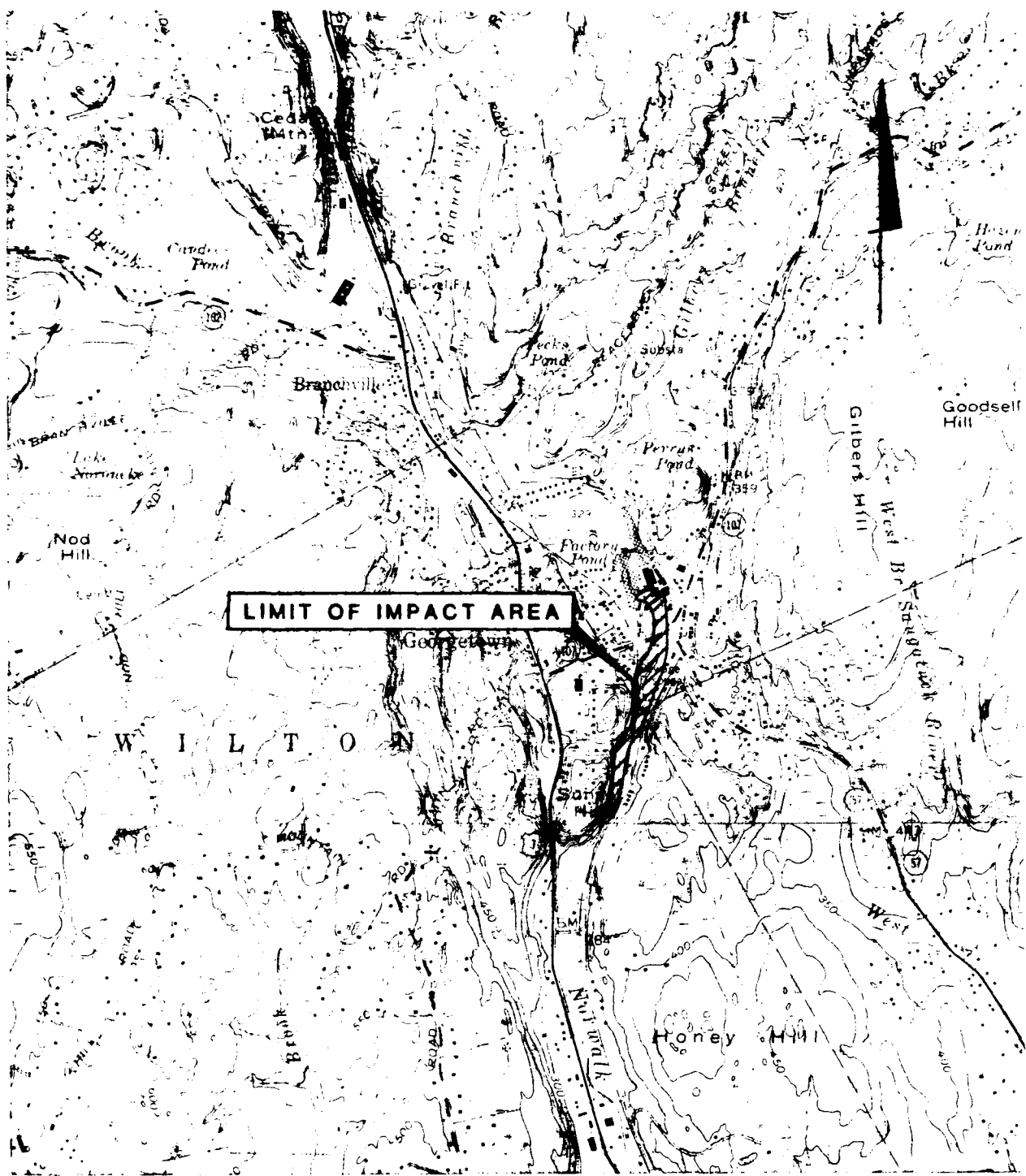


PLATE 4

| | | | |
|--|--|--|----------------|
| STORCH ENGINEERS WETHERSFIELD,CONNECTICUT | | US ARMY ENGINEER DIV NEW ENGLAND CORPS OF ENGINEERS WALTHAM MASS | |
| NATIONAL PROGRAM OF INSPECTION OF NON-FED DAMS | | | |
| FACTORY POND DAM | | | |
| | | | SCALE AS SHOWN |
| | | | DATE JULY 1980 |

scale 1:24000

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB Phase I Dam Inspection - #4463
SHEET NO 1 OF 7
CALCULATED BY GJG DATE 7/11/86
CHECKED BY ELG DATE 7/15/86
Determination of Test Flood

NAME OF DAM Factory Pond Dam

DRAINAGE AREA 12.2 SM

INFLOW $\frac{1}{2} PMF = \frac{1}{2} (1580 \times 12.2) = 9638 \text{ cfs}$

Estimating the effect of surcharge storage on the Maximum Probable Discharges

1. $Q_{p1} = \underline{9640} \text{ cfs}$
2a. $H_1 = \underline{9.8'} \text{ (elev.)}$
b. $STOR_1 = \underline{0.315''}$
c. $Q_{p2} = Q_{p1} (1 - STOR_1/19) = \underline{9480} \text{ cfs}$
3a. $H_2 = \underline{9.6'}$ $STOR_2 = \underline{0.30}$
b. $STOR_A = \underline{0.307}$
 $Q_{PA} = \underline{9330} \text{ cfs}$
 $H_A = \underline{9.7''}$ $STOR_A = \underline{0.307}$

Test Flood = 9330 cfs

Capacity of the spillway when the pond elevation is at the top of the dam

$Q = \underline{2500} \text{ cfs}$ or 27 % of the Test Flood

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB Phase I Dam Inspection 4463

SHEET NO. 1 OF 1

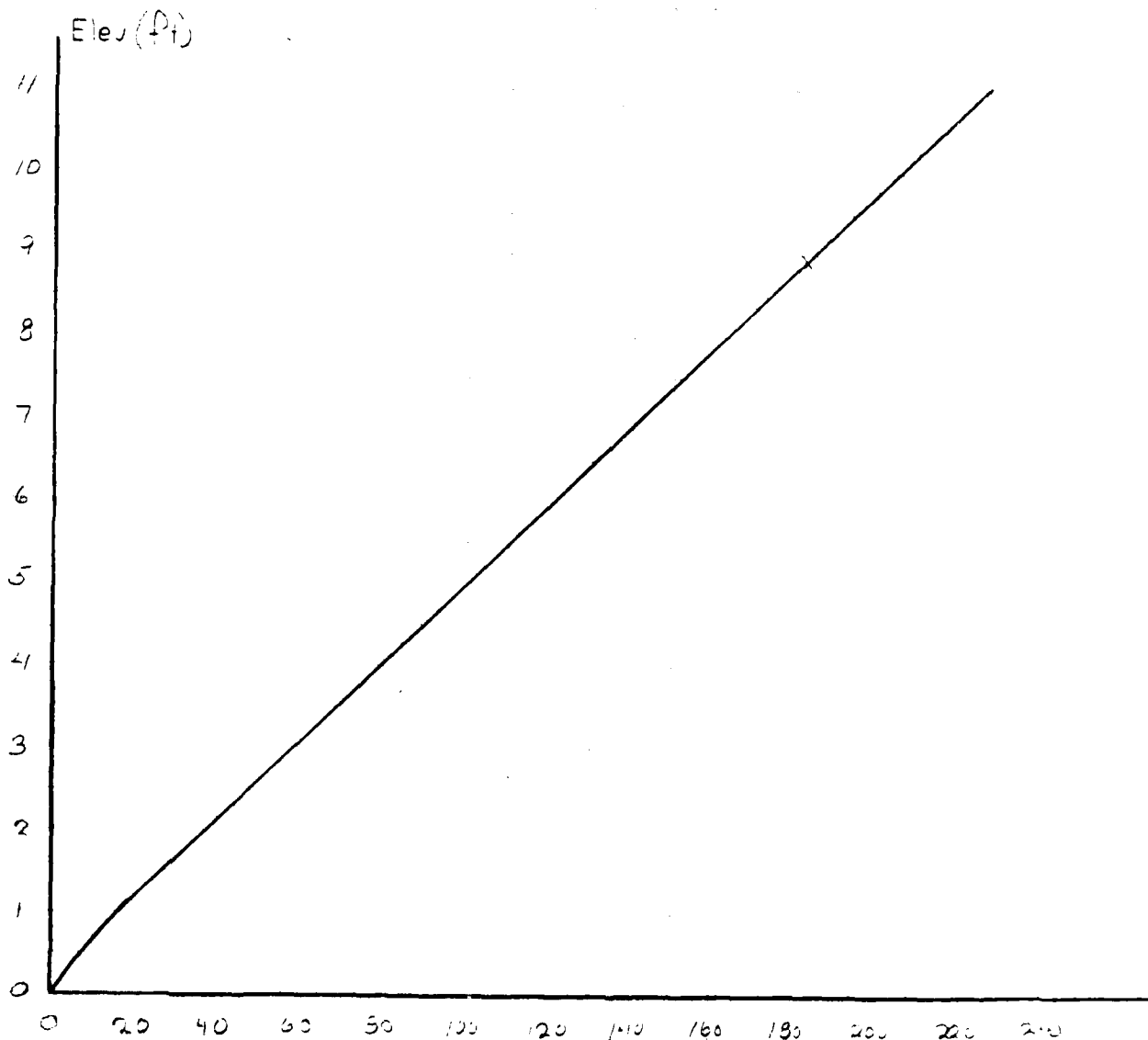
CALCULATED BY GJG DATE 7/16/80

CHECKED BY PKC DATE 7/15/80

AREA - CAPACITY

Name of Dam: FACTORY POND DAM

| ELEV | DEPTH | AREA | AVG. AREA | VOL | Σ VOL |
|------|-------|------|-----------|------|-------|
| 0.0 | | 16.5 | | | 0.0 |
| 1.0 | 1.0 | 17.0 | 16.8 | 16.8 | 16.8 |
| 11.0 | 10.0 | 24.8 | 20.9 | 209 | 226 |



STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB Phase I Dam Inspection 4463

SHEET NO. 2 OF 7

CALCULATED BY KJP DATE 4/24/80

CHECKED BY EJC DATE 7/5/80

SCALE Stage Discharge

NAME OF DAM FACTORY

$$Q = CLH^{3/2}$$

Assumed
100 cfs

| Elev | Spillway I | | | | Spillway II | | | | Dam | | | | QT |
|------|------------|------|------|---|-------------|---|---|---|------|-----|------|------|----|
| | C | L | H | Q | C | L | H | Q | C | L | H | Q | |
| 2.61 | 75' | 0.5 | 69 | | | | | | 2.68 | 60' | .35 | 33 | |
| 2.66 | | 1.0 | 199 | | | | | | 2.68 | | .45 | 126 | |
| 2.78 | | 1.5 | 383 | | | | | | 2.67 | | 1.35 | 340 | |
| 2.85 | | 2.0 | 605 | | | | | | 2.66 | | 1.45 | 401 | |
| 3.87 | | 2.5 | 850 | | | | | | 2.72 | | 2.35 | 587 | |
| 3.20 | | 3.0 | 1250 | | | | | | 2.73 | | 2.85 | 706 | |
| 3.32 | | 3.5 | 1630 | | | | | | 2.76 | | 3.35 | 1015 | |
| | | 4.0 | 1992 | | | | | | 2.79 | | 3.85 | 1265 | |
| | | 4.5 | 2377 | | | | | | 2.88 | | 4.35 | 1568 | |
| | | 4.65 | 2497 | | | | | | | | | | |
| | | 5.0 | 2789 | | | | | | | | | | |
| | | 5.5 | 3212 | | | | | | | | | | |
| | | 6.0 | 3660 | | | | | | | | | | |
| | | 6.5 | 4126 | | | | | | | | | | |
| | | 7.0 | 4612 | | | | | | | | | | |
| | | 7.5 | 5114 | | | | | | | | | | |
| | | 8.0 | 5634 | | | | | | | | | | |
| | | 8.5 | 6173 | | | | | | | | | | |
| | | 9.0 | 6725 | | | | | | | | | | |

Depth
10

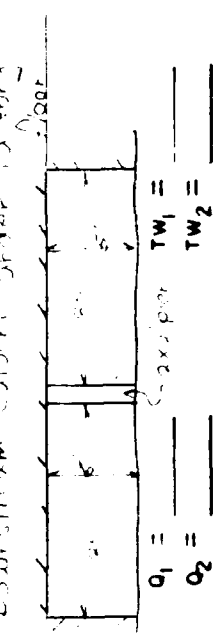
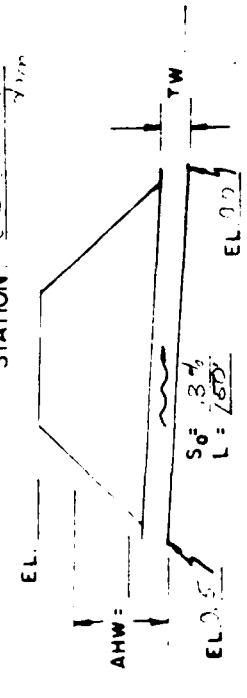
Total

Top of Dam E1933.5

4.65 ft

Elev 329.0

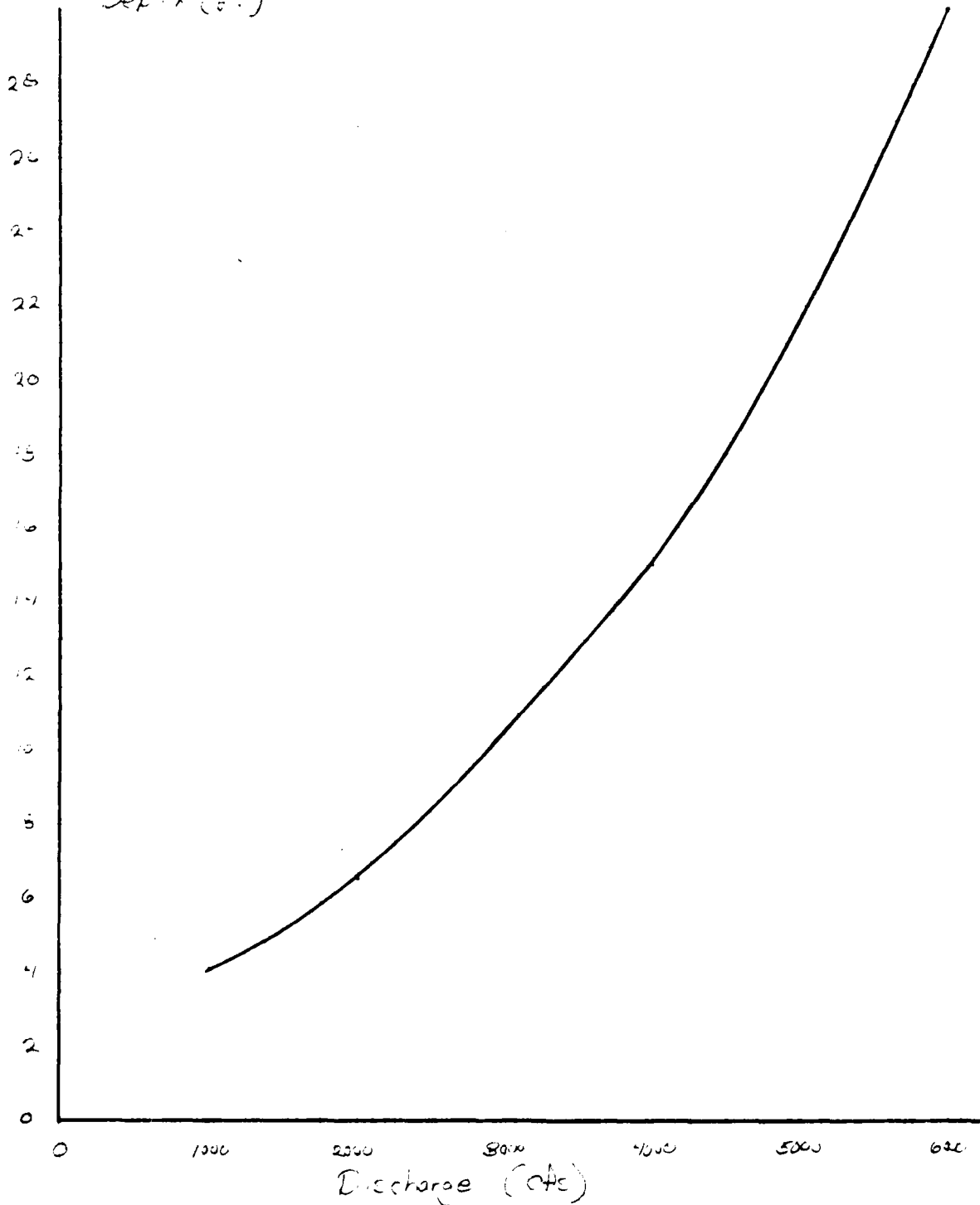
4/2

| <u>Project Factory Pond Dam</u> Designed By: <u>RLR</u> Date: <u>7/2/93</u> Town: <u>Georgetown</u> Route: <u>1</u> Checked By: <u>RLR</u> Date: <u>7/15/93</u> | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|--|--|--|-----------------------|----------------|-----|----------------|--------------------------------|-----|----------------|----------|------|----------|-----------------|-----------------|----------|------------|----------------|----|----|----------------|---|----------------|--------------------------------|----|----------------|-----------------|-----------------|-----|-----|-----|-----|---|-----|-----|-----|-----|--|--|------|-----|-----|-----|-----|-----|--|-----|-----|-----|-----|--|--|-----|-----|-----|-----|-----|-----|--|-----|-----|-----|-----|--|--|-----|------|-----|-----|-----|-----|--|-----|-----|-----|-----|--|--|------|------|-----|-----|-----|-----|--|-----|-----|-----|-----|--|--|------|------|
| HYDROLOGIC AND CHANNEL INFORMATION Downstream Culvert under Factory Pond  <p>Q1 = <u>100</u> TW1 = <u>10</u> Q2 = <u>100</u> TW2 = <u>10</u></p> <p>C = DESIGN DISCHARGE, SAY Q25 (Q2 = CHECK DISCHARGE, SAY Q50 OR Q100)</p> | SKETCH STATION: <u>50' from dam</u>  <p>MEAN STREAM VELOCITY = <u> </u> MAX. STREAM VELOCITY = <u> </u></p> | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| CULVERT DESCRIPTION (ENTRANCE TYPE) 500 1000 1500 2000 3000 | SIZE 5' x 2' 10' x 10' 15' x 15' 20' x 20' 30' x 30' | <table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th colspan="10">HEADWATER COMPUTATION</th> <th rowspan="2">VELOCITY</th> <th rowspan="2">COST</th> <th rowspan="2">COMMENTS</th> </tr> <tr> <th>INLET CONT</th> <th>OUTLET CONTROL</th> <th>HW</th> <th>HW</th> <th>K_a</th> <th>H</th> <th>d_c</th> <th>d_c³/2</th> <th>TW</th> <th>h₀</th> <th>LS₀</th> <th>HW₀</th> </tr> </thead> <tbody> <tr> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>5</td> <td>4.5</td> <td>2.3</td> <td>3.7</td> <td>3.7</td> <td></td> <td></td> <td>3.65</td> <td>4.1</td> </tr> <tr> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td></td> <td>1.3</td> <td>3.7</td> <td>4.2</td> <td>4.3</td> <td></td> <td></td> <td>5.5</td> <td>4.5</td> </tr> <tr> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td></td> <td>1.3</td> <td>4.8</td> <td>4.9</td> <td>4.2</td> <td></td> <td></td> <td>6.1</td> <td>10.5</td> </tr> <tr> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td></td> <td>1.3</td> <td>5.0</td> <td>5.0</td> <td>5.0</td> <td></td> <td></td> <td>12.0</td> <td>15.0</td> </tr> <tr> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td>1.3</td> <td></td> <td>1.3</td> <td>5.0</td> <td>5.0</td> <td>5.0</td> <td></td> <td></td> <td>20.5</td> <td>20.0</td> </tr> </tbody> </table> | HEADWATER COMPUTATION | | | | | | | | | | VELOCITY | COST | COMMENTS | INLET CONT | OUTLET CONTROL | HW | HW | K _a | H | d _c | d _c ³ /2 | TW | h ₀ | LS ₀ | HW ₀ | 1.3 | 1.3 | 1.3 | 1.3 | 5 | 4.5 | 2.3 | 3.7 | 3.7 | | | 3.65 | 4.1 | 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 3.7 | 4.2 | 4.3 | | | 5.5 | 4.5 | 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 4.8 | 4.9 | 4.2 | | | 6.1 | 10.5 | 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 5.0 | 5.0 | 5.0 | | | 12.0 | 15.0 | 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 5.0 | 5.0 | 5.0 | | | 20.5 | 20.0 |
| HEADWATER COMPUTATION | | | | | | | | | | VELOCITY | COST | COMMENTS | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| INLET CONT | OUTLET CONTROL | HW | HW | K _a | H | d _c | d _c ³ /2 | TW | h ₀ | | | | LS ₀ | HW ₀ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.3 | 1.3 | 1.3 | 1.3 | 5 | 4.5 | 2.3 | 3.7 | 3.7 | | | 3.65 | 4.1 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 3.7 | 4.2 | 4.3 | | | 5.5 | 4.5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 4.8 | 4.9 | 4.2 | | | 6.1 | 10.5 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 5.0 | 5.0 | 5.0 | | | 12.0 | 15.0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1.3 | 1.3 | 1.3 | 1.3 | | 1.3 | 5.0 | 5.0 | 5.0 | | | 20.5 | 20.0 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| SUMMARY & RECOMMENDATIONS | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB 14163
SHEET NO 5 OF 9
CALCULATED BY G.J.G. DATE 7/3/80
CHECKED BY RDC DATE 7/15/80
SCALE _____

Downstream Culvert Rating Curve
Depth (ft)



D-4

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB Phase I Dam Inspection - #4463

SHEET NO. 6 OF 7

CALCULATED BY KLP DATE 7/3/80

CHECKED BY PCC DATE 7/15/80

Downstream Hydrographs

"Rule of Thumb" Guidance for Estimating Downstream Failure Hydrographs

NAME OF DAM FACTORY

Section I at Dam

1. $S = \frac{192}{\text{Acft}}$
2. $Q_{p1} = 8/27 W_b \sqrt{9} Y^{3/2} = 8/27 (30) \sqrt{32.2} (23.4)^{3/2} = 5709 \text{ cfs}$
3. See Sections

Section II at

- 4a. $H_2 = 8.9$ $A_2 = 225 \text{ ft}^2$ $L_2 = 800$ $V_2 = 4.1 \text{ Acft}$
- b. $Q_{p2} = Q_{p1} (1 - V_2/S) = 5587 \text{ cfs}$
- c. $H_2 = 8.9$ $A_2 = 225$
 $A_A = 225 \text{ ft}^2$ $V_2 = 4.1 \text{ Acft}$
 $Q_{p2} = 5709 (1 - 4.1/92) = 5587$

Section III at

- 4a. $H_3 = 7.0$ $A_3 = 275$ $L_3 = 1000$ $V_3 = 6.3 \text{ Acft}$
- b. $Q_{p3} = Q_{p2} (1 - V_3/S) = 5393 \text{ cfs}$
- c. $H_3 = 7.0$ $A_3 = 275$
 $A_A = 275$ $V_3 = 6.3 \text{ Acft}$
 $Q_{p3} = 5587 (1 - 6.3/188) = 5400$

Section IV at

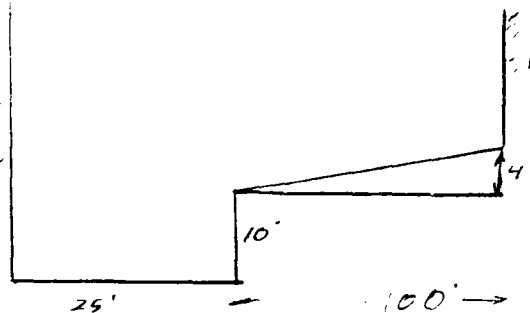
- 4a. $H_4 = 7.0$ $A_4 = 530$ $L_4 = 2100$ $V_4 = 26.0' \text{ Acft}$
- b. $Q_{p4} = Q_{p3} (1 - V_4/S) = 4630 \text{ cfs}$
- c. $H_4 = 6.5$ $A_4 = 500$
 $A_A = 515$ $V_4 = 25.0' \text{ Acft}$
 $Q_{p4} = 5400 (1 - 25/182) = 4660$

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

JOB 4-133
SHEET NO 6 OF 9
CALCULATED BY KLD DATE 7/3/80
CHECKED BY 226 DATE 5/15/80
SCALE SECTION 1L

Flow 800'

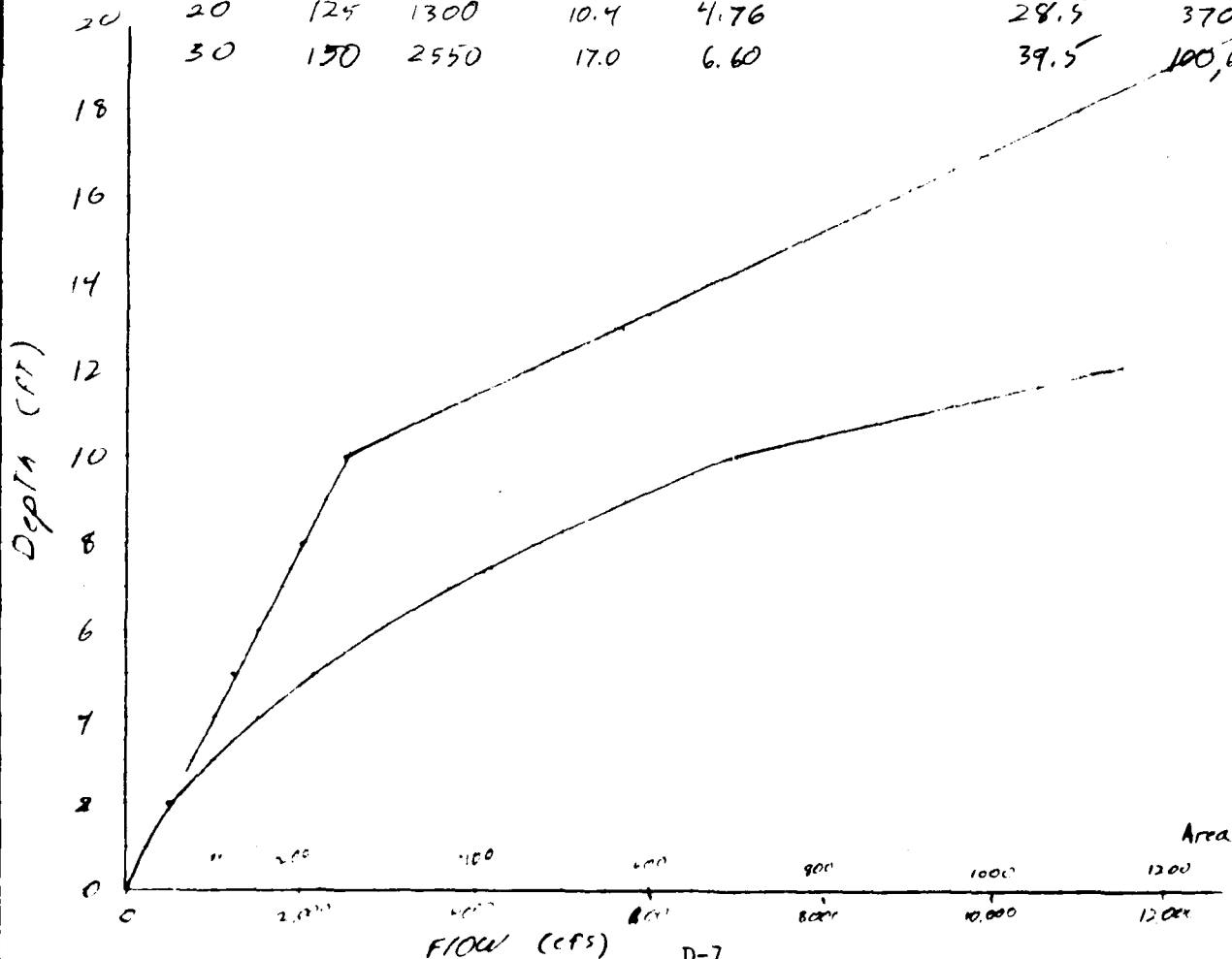
Buildings



Buildings

$n = .035$
 $s = 2\%$

| D | W ² | A | R | R ^{2/3} | S ^{1/2} | V | Q |
|----|----------------|------|------|------------------|------------------|------|---------|
| 2 | 25 | 50 | 2 | 1.59 | .1414 | 9.49 | 474 |
| 5 | 25 | 125 | 5 | 2.92 | | 17.5 | 2183 |
| 8 | 25 | 200 | 8 | 3.99 | | 23.9 | 4777 |
| 10 | 25 | 250 | 10 | 4.63 | | 27.7 | 6928 |
| 20 | 125 | 1300 | 10.4 | 4.76 | | 28.5 | 37034 |
| 30 | 150 | 2550 | 17.0 | 6.60 | | 39.5 | 100,630 |



D-7

STORCH ENGINEERS
Engineers - Landscape Architects
Planners - Environmental Consultants

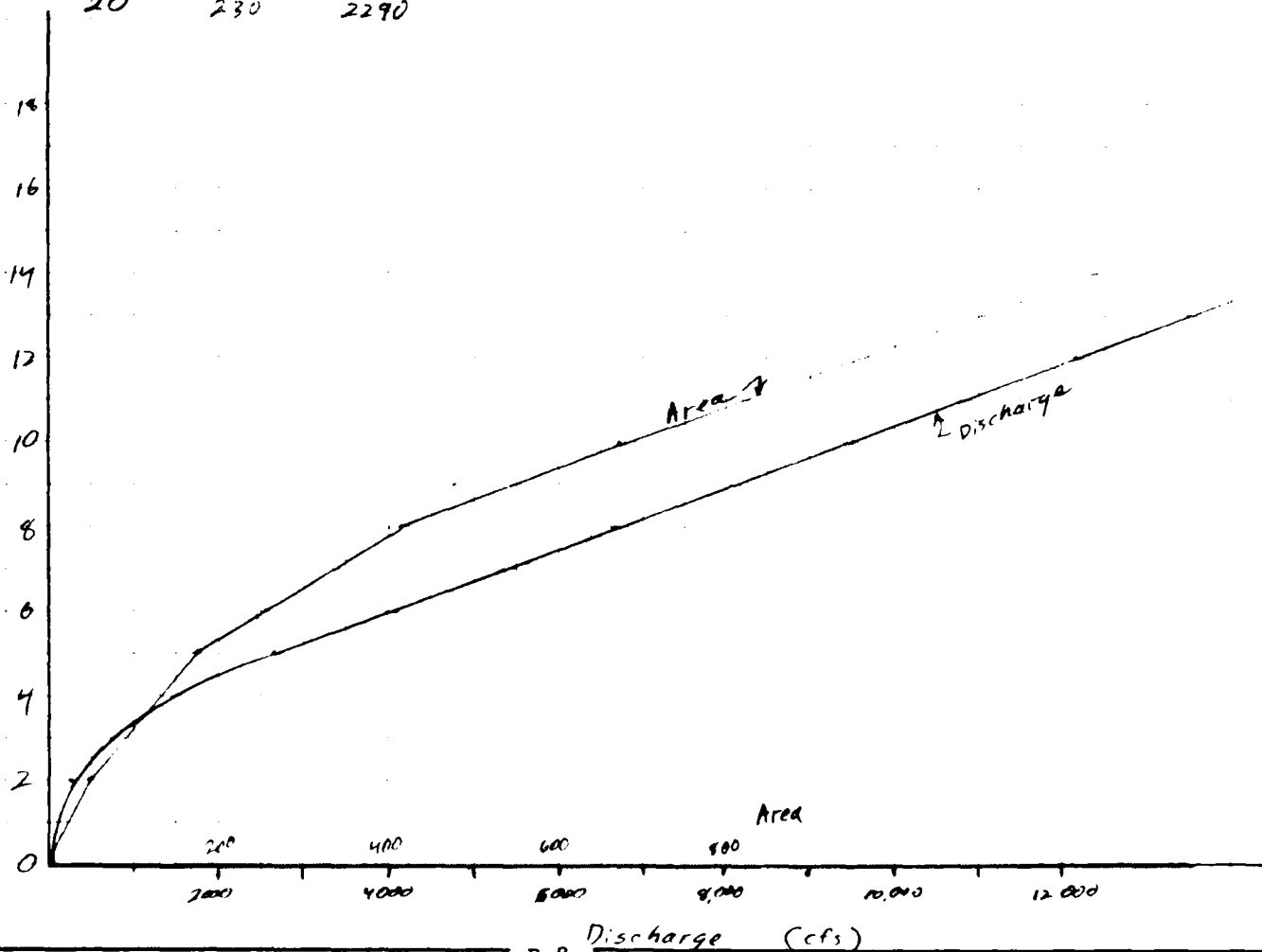
JOB 53
SHEET NO 8 OF 9
CALCULATED BY KJP DATE 7/2/80
CHECKED BY 3DC DATE 5/15/80
SCALE SECTION III

800 → 1800' Down stream



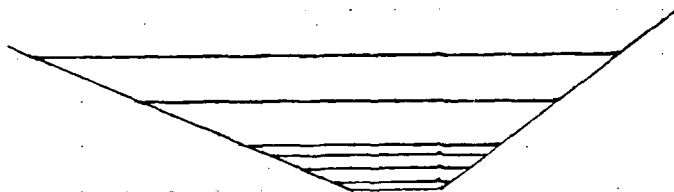
slope = 5%
n = .05

| D | WP | A | R | R ^{2/3} | S ^{1/2} | V | Q |
|----|-----|------|------|------------------|------------------|-------|-------|
| 2' | 30 | 50 | 1.66 | 1.405 | .2236 | 9.30 | 280 |
| 5 | 50 | 125 | 3.50 | 2.303 | | 15.24 | 2667 |
| 8 | 110 | 418 | 3.77 | 2.421 | | 16.02 | 6650 |
| 10 | 150 | 675 | 4.50 | 2.725 | | 18.07 | 12176 |
| 20 | 230 | 2290 | | | | | |



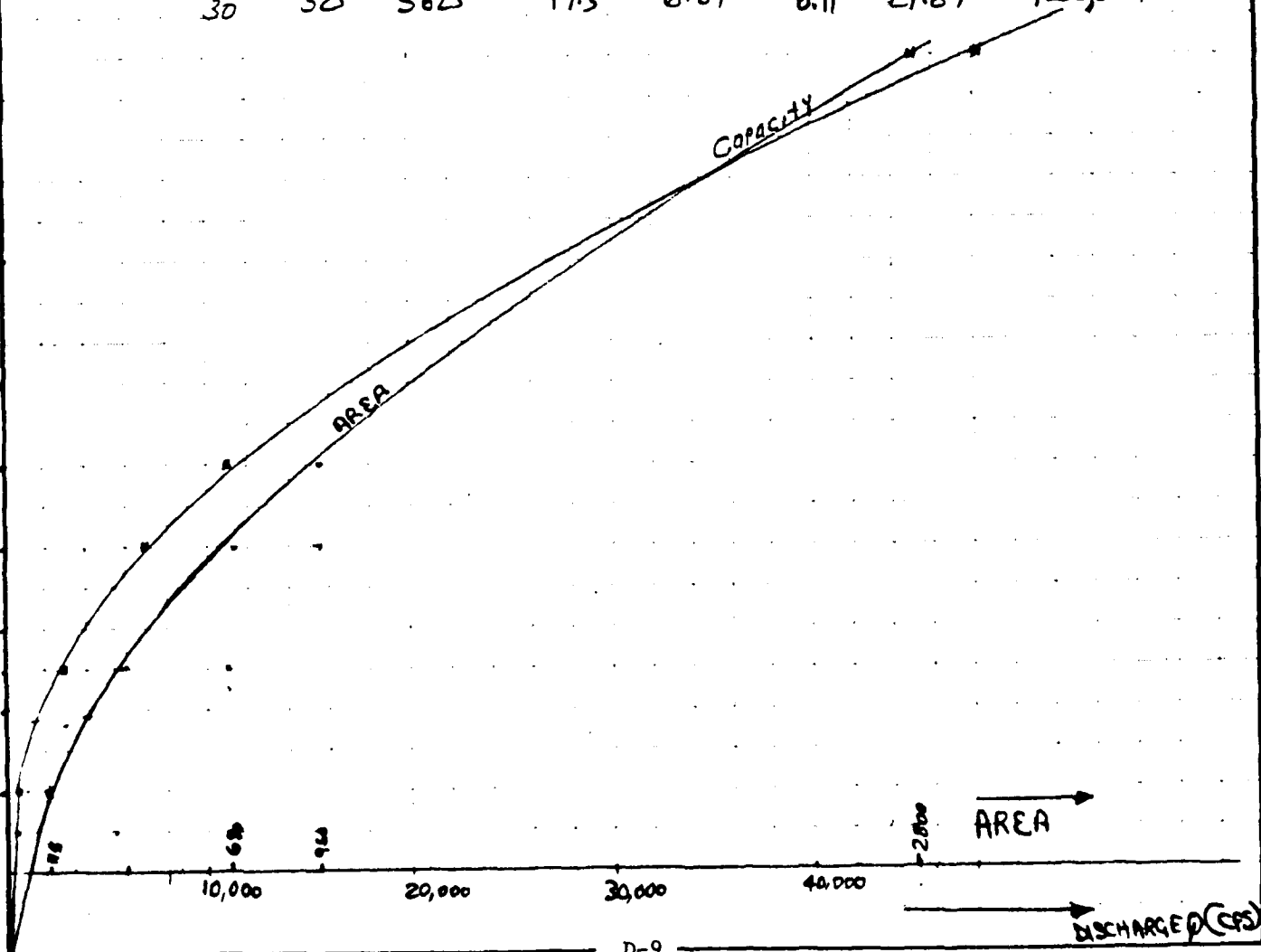
STORCH ENGINEERS/STORCH ASSOCIATES
 Engineers - Landscape Architects
 Planners - Environmental Consultants

JOB 4-103
 SHEET NO 9 OF 9
 CALCULATED BY K.P. DATE 7/3/80
 CHECKED BY BDC DATE 7/15/80
 SCALE Section IV



$n = .05$
 $s = 1.25\%$

| D | WP | A | R | R^2 | $S^{\frac{1}{2}}$ | V | Q |
|----|-----|------|-------|-------|-------------------|-------|--------|
| 2 | 65 | 115 | 1.77 | 1.46 | 0.11 | 4.77 | 549 |
| 5 | 93 | 358 | 3.85 | 2.46 | 0.11 | 8.09 | 2879 |
| 8 | 120 | 680 | 5.67 | 3.18 | 0.11 | 10.39 | 7069 |
| 10 | 140 | 950 | 6.79 | 3.58 | 0.11 | 11.70 | 11119 |
| 20 | 230 | 2800 | 12.17 | 5.29 | 0.11 | 17.29 | 48423 |
| 30 | 325 | 5625 | 17.3 | 6.69 | 0.11 | 21.87 | 123024 |



APPENDIX E

INFORMATION AS CONTAINED IN
THE NATIONAL INVENTORY OF DAMS

INVENTORY OF DAMS IN THE UNITED STATES

| | | | | | | | | |
|-------|----------|----------|----------|-------|--------|-------|------------------|-------------|
| STATE | IDENTITY | DIVISION | CONCRETE | STATE | COUNTY | DIST. | NAME | REPORT DATE |
| CT | 217 | NEC | 00 | 00 | | | FACTORY POND DAM | 08 JAN 74 |

| | |
|-----------------|--|
| POPULAR NAME | NAME OF IMPONDMENT |
| | FACTORY POND |
| RIVER OR STREAM | NEAREST DOWNSTREAM CITY - TOWN - VILLAGE |
| NORWALK RIVER | GEORGETOWN |
| TYPE OF DAM | POPULATION |
| 1 | 1900 |

| | | | | | |
|-------------|----------------|----------|-----------------------|------------------------|----------------------------------|
| TYPE OF DAM | YEAR COMPLETED | PURPOSES | STILLING HEIGHT (FT.) | HYDRAULIC HEIGHT (FT.) | IMPOUNDING CAPACITIES (ACRE-FT.) |
| 1 | 1874 | 0 | 21 | 19 | 157 |

| |
|---|
| REMARKS |
| ESTIMATE 21-GROUTED STONE 22-ESTIMATED 23-FACTORY USE |

| | | | | |
|----------|-------------------------|-----------------------|---------------------|------------------|
| SPILLWAY | MAXIMUM DISCHARGE (CFS) | VOLUME OF DAM (CU YD) | POWER CAPACITY (KW) | NAVIGATION LOCKS |
| 1 | 175 | U 75 | 2500 | |

| | | |
|-------------------|------------------|-----------------|
| OWNER | ENGINEERING BY | CONSTRUCTION BY |
| Gilbert & Bennett | Industrial Assoc | |

| | | | |
|--------|--------------|-----------|-------------|
| DESIGN | CONSTRUCTION | OPERATION | MAINTENANCE |
| | | | |

| | | |
|------------------|-----------------|--------------------------|
| INSPECTION BY | INSPECTION DATE | AUTHORITY FOR INSPECTION |
| Storch Engineers | 23 Apr 80 | PL 92-367 |

| |
|------------------------|
| REMARKS |
| 47-1956 Reconstruction |

DIST OWN FED R PRV/FED SCS A VER/DATE

END

FILMED

8-84